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AN INVESTIGATION OF THE COMPRESSIVE STRENGTH PROPERTIES OF STAINLESS STEEL SHEET-STRINGER COMBINATIONS



Air Service Information Circular, Volume VII, No. 697

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Air Service Engineering Division McCook Field Dayton OH 45430

November 30, 1934

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(AIRCRAFT BRANCH REPORT)



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AN INVESTIGATION OF THE COMPRESSIVE STRENGTH PROPERTIES OF STAINLESS STEEL SHEET-STRINGER COMBINATIONS

(Prepared by E. H. Schwartz and C. G. Brown, Matériel Division, Air Corps, Wright Field, Dayton, Ohio, Sept. 10, 1934)

SUMMARY

An examination of the test data indicates that-

1. The strength properties of sheet stringer combinations are governed principally by the strength properties of the stringer, and only to a minor degree by the sheet to which the stringer is attached, and that the highest structural efficiency from the point of view of carrying compressive loads is attained for a given stiffener by reducing the skin thickness to a minimum.

2. For a sheet-stringer combination the total load carried is a linear function of the stiffener units provided, and is independent of the stiffener pitch.

3. The effective width of sheet working with a stiffener at any stiffener stress, up to the stiffener failing stress for a given sheet thickness may be determined by the expression:

$$W = C \sqrt{\frac{E}{\sigma_{\bullet}}} t,$$

the units being defined in the body of the report.

4. The coefficient C in the foregoing expression is a function, primarily, of sheet thickness. The data were insufficient to definitely indicate any dependence of C on stiffener failing stress, for a given sheet thickness.

5. Having once established a curve of stiffener σ_s versus L/ρ , the properties of any sheet-stiffener combination may be determined directly.

6. The properties of stainless steel columns follow a form of Euler-Johnson relationship, the position and shape of the curve for a particular section being a function of the cross section shape and thickness of that section and of the column fixity.

7. The properties of corrugated stainless steel sheets are governed by both slenderness ratio and R/t ratio, in such a manner that buckling values for a certain R/t ratio decrease with increasing L/ρ .

8. For a given stiffener shape and length its failing stress increases with thickness up to some limiting thickness after which it remains constant.

9. For a constant stiffener shape, thickness, and length, the load per stiffener in sheet-stiffener combinations increases parabolically with increase in sheet thickness.

10. For such data as were available the linear relationship between load carried and stiffener units, and independence from stiffener pitch applies as well to aluminum alloy plate stringer combinations.

11. The present specification for the strength of spot welds is not adequate to prevent failures of the type experienced during these tests. It is felt that it should be amended by a requirement to the effect that the weld strength of built-up sections be demonstrated by test to be sufficient to allow complete collapse, or crushing, of the section without weld failures.

HISTORY

The project was initiated in October 1931 and drawings and procurement data were completed by June 1932 at which time invitations for bids were issued.

The bids received at that time were so in excess of the funds available to carry out the project that the stiffener sections were changed from the original section which had no outstanding welding flanges at the back of the stiffener to the type of stiffener shown herein, in order to reduce the cost of the specimens to a point where procurement was possible with available funds.

New invitations were then issued and eventually a contract awarded to the Curtiss Aeroplane & Motor Co. for their fabrication.

The selection of stiffener cross section, gages, etc. was influenced by and based upon the data available at the time the project was initiated, namely 1931.

Certain sections presented herein, however, were decided upon at a later date to supply information of a specific nature, and are included herein to broaden somewhat the scope of the original project.

DATES AND PLACE OF TESTS

All tests were conducted between December 1933, and June 1934, as time and equipment were available. The majority of the tests were conducted at Wright Field, Dayton, Ohio, using the testing equipment of the Materials Branch. Tests on specimens requiring a load in excess of 100,000 lb./sq. in. to cause failure were tested at the Bureau of Standards, Washington, D. C.

OBJECT

The object of the project was to determine, insofar as possible, the effects of the many variables that enter into the determination of the compressive strength properties of the flat stainless steel sheet-stiffener combinations tested during the course of the investigation.

DESCRIPTION

Test specimens

The specimens were fabricated in accordance with Matériel Division drawings SK-18698, 18699, and 533064 from commercial 18-8 sheets, no Army specification on stainless steel being available at the time the speciments were made. The physical properties of the material, as determined by tests on specimens of representative thicknesses taken from the specimens, are tabulated in table I.

The sheet-stiffener combinations consisted of closed section stiffeners of the type shown in figures 1 and 2, electrically spot welded to sheets. The spot welding technique was determined by the practice of the contractor. The weld spacings were as follows:

Sheet thick-	Weld
ness	spacing
Inch	Inch
0. 010-0. 015	316
. 020 030	34
. 050	38

The welding flanges were $\%_6 \pm \%_4$ on thicknesses of 0.010 to 0.030, and $\%_4 \pm \%_4$ on 0.050 sheets.

The allowable tolerance on the sheets were:

Sheet thick- ness	Tolerance
Inch	Inch
0. 010-0. 030	±. 015
. 031 051	±. 0025

The tolerance on the stiffener over-all dimensions was $\pm \frac{1}{164}$ inch and on spacing was $\pm \frac{1}{162}$ inch.

The allowable deviation from straightness was set at not to exceed $\frac{1}{2}$ inch per foot of specimen length. The over-all length of the specimens was held to $\pm \frac{1}{16}$ inch of that specified.

The ends of the specimens were ground flat and were liquid-cooled during grinding. A departure from parallel ends to the extent of 0.004 per inch width or depth of specimen or stiffener was allowed.

It proved practically impossible to secure perfectly flat ends on the specimens, and accordingly the sheet was allowed to depart 0.003 inch from flat, at a point midway between stiffeners, on sheets of 0.010 inch thickness, and 0.002 inch from flat at the midpoint on sheets of 0.050 inch thickness. Some of the first specimens received, that is, I series, were reground at the division to obtain flatter ends. The set-up used for regrinding is shown in figure 79.

A reasonable degree of flatness of the skin between stiffeners was required, and a satisfactory flatness was

furnished in practically all specimens. A twist of the finished specimens of not to exceed 0.01 inch per inch specimen length was allowed. This twist was usually readily removable at the time of testing.

The test series were divided into several parts, as follows:

Parts A and B.—The specimen length was held constant, and the stiffener thickness, sheet thickness, and stiffener pitch, and number of stiffeners varied in order. The stiffener cross section dimensions were held constant. Section shown in figures 1 and 78.

Part C.—In this series length effect was studied by controlling lengths for certain stiffener and sheet combinations. The stiffener cross section was held constant, being the same as the A series stiffener section. Five lengths each were arranged.

Part EA.—In this series the stiffener width was held constant, and the stiffener depth varied for several combinations of sheet and stiffener thicknesses, and for three lengths each. See figures 2 and 78 for typical stiffener section.

Part EB.—In this case the stiffener depth was held constant and the width varied, other factors being controlled as in part EA. See figures 2 and 78 for typical stiffener section.

Part EC.—In this series, both width and depth were allowed to vary, the other variables being as in part EA. Typical stiffener section shown in figures 2 and 78.

Part F.—This series was essentially as in EA, except that the method of attaching the stiffeners to the sheet involved four rows of welds on the sheet instead of two and that additional stabilizing grooves, spaced ½ inch, were provided in the stiffener sides. A section is shown in figure 2.

Part G.—A series of stainless steel cylinders of varying stiffener spacings and thicknesses for varying sheet thicknesses, lengths, and cylinder diameters. Stiffener section constant as in part A. (Not tested at date of writing.)

Part H.—A series of flanged angle section were used for stiffeners in varying depths, lengths, skin and stiffener thicknesses, and stiffener spacing. (Note.—Specimens were not tested, due to their inefficient nature.)

Part I.—Here the specimens consisted of corrugations of ½ inch depth, with the pitch of the corrugations varying for certain thickness and lengths. Various flat sheets welded to the corrugations to simulate wing covering were also investigated. Typical section shown in figure 3.

Description of apparatus

In order to eliminate any undesirable effects due to lateral shifting of testing machine tests, the jigs shown in figures nos. 74, 75, and 76 were used. The jig consisted essentially of four 8- by 8- by 2-inch steel plates, having alternate plates rigidly connected by accurately ground shafts sliding through bronze bearings in adjacent blocks in such a manner that a compressive load is applied between the inner blocks when a tensile load is applied to the outer blocks. Due to the close, sliding fit between the shafts and the blocks lateral movement of one end of a test specimen with respect to the other end was eliminated.

Shafts of sufficient length to accommodate the longest specimens were provided. A close-up view of a long specimen in the jig is shown in figure no. 76.

A 2-inch square bar, ground flat, was secured to the upper working block of the jig, and a 3-inch square bar, likewise ground flat, centered on a spherical seating block centered on the lower working block.

Strips of hardened aluminum alloy 2 inches in width, and ½ inch thick were placed between the ends of the specimens and the ground steel surfaces, in order to prevent damage and to assist in attaining a more uniform load distribution. Figure no. 77 shows typical impressions of specimens left on the strips after testing.

With the jig installed in a testing machine, the specimens were then loaded to failure, operating the machine as if for a tensile test.

A 40,000-pound capacity Amsler and a 100,000-pound Olsen machine were used at Wright Field. A 600,000-pound Olsen machine at the Bureau of Standards was used to load specimens exceeding the capacity of the other two machines.

It was noted during the tests that considerable energy was stored in the guide rods of the testing jig. When a specimen began to fail, the stored energy was often sufficient to cause the failure to proceed to completion in an explosive manner, which was somewhat undesirable for the study of the process of failure.

Areas were determined from the actual weight and length of the individual specimens, using as the unit weight of stainless steel 0.286 lb./cu. in., which was an average value determined by weighing several sheets of known dimensions.

DISCUSSION

A series

The variables that may be investigated in the A series of specimens are, for a constant stiffener shape and length, as follows:

- 1. Stiffener thickness (0.010 to 0.050) normal thickness.
 - 2. Sheet thickness (0.010 to 0.050) normal thickness.
 - 3. Stiffener spacing (1% to 15 inches).
 - 4. Number of stiffeners (2 to 9).

The actual thicknesses were close to 0.009, 0.014, 0.018, 0.019, 0.029, and 0.049. During the planning stages of the project it was expected that a useful variable might be found in percent reinforcement defined by the expression:

$$R = \frac{\text{Stiffener area}}{\text{Stiffener area} + \text{sheet area}}$$

where the stiffener area is that of one stiffener, and the sheet area is that included between the centers of the adjacent stiffeners.

The variable R, in the case of a specimen having but two stiffeners, obviously gives an erroneous indication of the existing conditions, and is of questionable value when applied to two stiffener test specimens. Accordingly, little faith has been placed on values indicated by many two-stiffener specimens.

Average failing stresses have been plotted against R in figures 18 to 22, inclusive. If the range of R is

investigated, it will be noted that the average failing stress varies linearly with R, the position and slope of the resulting line being determined, for a given stiffener, by the skin thickness. The effect of an increase in stiffener thickness or sheet thickness for a given R is to increase the average failing stress.

In reference 8, it is concluded that the optimum conditions for proportioning a stiffened sheet is that in which the stiffener retains its alinement until the sheet is carrying the maximum load of which it is capable.

This conclusion is apt to be misleading, for it emphasizes the sheet as the more important element of the sheet-stringer combination. It unquestionably applies to the case of a weak stiffener attached to a thick sheet, which is an inefficient combination in comparison to the reverse case.

In cases of sheet-stiffener combinations of normal proportions, it is quite possible for stiffeners of closed section to remain intact and to continue to carry high stresses long after the sheet has reached and pressed its maximum load. Even in cases where a more nearly simultaneous failure of the elements occurs, the portion of the total load carried by the sheet is small in contrast to that carried by the stiffeners. As stiffener thickness increases, the average failing stress goes up, and the percentage of load carried by the sheet goes down. In general, it may be noted that as the load carried by the sheet increases, the average failing stress lowers.

Accordingly, it would appear that to obtain the highest average failing stresses, the stiffener should be the element that carries the greatest emphasis, and that the sheet should be governed by conditions other than its simple load-carrying ability.

It should be noted that the R curves of figures 18 to 22, inclusive, are for a particular stiffener section thickness and length and cannot be applied directly to any other section. Any variation in allowable stress as determined from an R curve must be due only to a change in stiffener spacing and not to a change in stiffener area. The reason being that the stiffener is by far the governing variable of the combination.

This may be readily noted by examination of figures 13 to 17, inclusive, and 8 to 12, inclusive, where in one case average failing stresses have been plotted against stiffener pitch for a constant stiffener and a varying sheet thickness, and next for a constant sheet and varying stiffener thickness. In the first instance little effect of the sheet is noted. In the second case, however, an enormous variance in average stress due to a change in stiffener thickness is noted.

When plotting the failing loads of the specimens against the number of stiffeners, as in figures 5 to 7, inclusive, a remarkably consistent variation is noted in that every combination of sheet and stiffener the total load carried is directly proportional to the number of stiffeners, regardless of their spacing. This variation is consistent with the findings of reference 1 where it was noted that the strength of flat plates under edge compression was independent of their width.

This immediately suggests that each stiffener and an unknown width of sheet form a unit working at the failing stress of the stiffener. It also explains the drop-

ping off of average failing stress with increased stiffener spacing indicated in figures 8 to 17, inclusive.

Due to the nature of the stiffeners, the assumption that the stiffener failing stress would remain constant with a change in sheet thickness appears to be justified. On the basis of that assumption, which will be discussed more fully later, an attempt will be made to determine the amount of sheet working with each stiffener at a particular stiffener failing stress.

Let

P=load, pounds, carried by the test specimen at failure, for N stiffeners.

 σ_s =failing stress of stiffener and whatever width of sheet may be working with it at failure. A_{sh} =effective area of sheet per stiffener.

The following relationship may be established in terms of the test specimen dimensions:

$$P = [NA_{st} + (N-1)A_{sh} + 0.875t]\sigma_s$$

Solving for Ash

$$\mathbf{A}_{sh} = \frac{P}{(N-1)\sigma_s} - \left(\frac{N}{N-1}\right) A_{st} - \frac{0.875t}{(N-1)}$$

The term 0.875t is included with the working area on the presumption, justified later, that the working width will be equal to or greater than the overall width of the stiffeners, and increases to 1.00t for 0.050 stiffeners.

In the above expression for A_{sh} the only unknown is σ_s . The definition of σ_s must be expanded to indicate a failing stress of a closed section where the closing sheet is of zero thickness, but still acts to supply the stabilizing forces necessary to prevent the stiffener from behaving as an open section.

In order to determine σ_s it was assumed that the effective width of sheet working with the stiffener was the overall width of the stiffener, % inch in the case of 0.029 stiffeners, and 1 inch in the case of 0.049 stiffeners. Assumed values of stress were then computed using a stiffener area including only the sheet between the stiffener overall dimensions, the number of stiffeners on a test specimen, and the specimen failing load.

These assumed stresses were then plotted against skin thickness and the resulting curves extrapolated to zero skin thickness to give a proper value of σ_* (curves not included).

In the calculations of σ_{\bullet} an average stiffener area has been used, together with values of P taken from P versus N curves. The calculations of σ_{\bullet} and $A_{\bullet h}$ are arranged in tabular form in tables II and III.

Figure 25 shows the variation of σ_* with stiffener thickness, and indicates a rapid increase of failing stress up to a thickness of about 0.030, where the curve abruptly flattens out. A similar behavior will be noted in figure 24 where average failing stresses for the B series stiffeners have been plotted against stiffener thickness for several sheet thicknesses.

In reference 4 will be found the expression-

$$W' = \frac{C}{2} \sqrt{\frac{E}{\sigma}} t$$

where

W' = one-half the width of sheet working with a stiffener

 $E = \hat{m}$ odulus of elasticity of material

 σ_{\bullet} =stiffener stress t=sheet thickness C=a constant W=2W'

Having computed the effective width of sheet working with a stiffener at a stress σ_s , it requires but two steps to compute W/t and C as has been done in table III, using the expression—

$$C = \frac{W}{\sqrt{\frac{E}{\sigma_a}}}$$

The coefficient C has been plotted in figure 26 against sheet thickness for σ_s from 71,500 lb./sq.in. to 120,000 lb./sq.in. Various stiffener thicknesses and lengths were used to obtain the σ_s variation.

The curves 1 and 2 of figure 26 indicate, respectively, the lower and upper limits of C as indicated by the plotted points. Curve 3 represents an estimated mean value of C and is the value recommended for use.

It will be noted from the computation of C that thickness enters as t^2 . Since it was not possible to maintain a constant t, as was noted under the description of the specimens, variations in t are felt responsible for a considerable part of the variation of figure 26. The greatest effect due to variation of t^2 would be noted where the thickness is small, and it is in this range where the results are most scattered.

The results are also influenced by the proper determination of σ_{\bullet} . Since σ_{\bullet} was necessarily determined by extrapolation, it is subject to the errors of that process.

Considering the sources of error involved, it is somewhat surprising that the computed values of C are not even more scattered than the results indicate.

C series

The purpose of this series was to investigate the influence of length for nominal sheet and stringer thicknesses of 0.010, 0.020, and 0.030, with spacings corresponding to certain of those used in the A series of specimens.

In figures 29 to 32, inclusive, the specimen failing load has been plotted against length for several sheet and stiffener thicknesses, for lengths up to 18 inches. The shape of the resulting curves is influenced considerably by the stiffener, and somewhat by the sheet.

In figures 27 and 28 a value of load per stiffener, P/N, obtained by dividing the total load carried by the number of stiffeners on the specimen, has been plotted. For the 0.019 and 0.029 stiffeners the resulting curves indicate that the linear relationship between total load and number of stiffeners, established for the A series, applies for any length within the range of lengths tested.

Due to the erratic results of the tests on the 0.009 stiffeners, the linear relationship is not experimentally so evident. The shape of the curves of P/N versus length is not completely consistent in shape, the curves for the 0.029 stiffener being least so.

The curves of σ_s versus stiffener thickness show the same trend toward a constant σ_s after a certain t is reached that was noted from the A and B series tests.

In figures 33 and 34, where average failing stresses have been plotted against length, an Euler-Johnson relationship is indicated.

Fixity effects

In order to obtain some idea of the fixity coefficient C realized during the flat end tests, a series of individual 0.029 stiffeners was salvaged from 0.019 sheets after the A and C series tests had been completed. The sheet was sheared off at the edges of the stiffener welding flanges.

The individual stiffeners were then tested as both knife-edge and flat-end columns. The results are presented in table IV and figure 36. On the latter figure Euler curves for C=1 and C=3, based on E=26,000,000, which is a fair average value, were plotted.

For the knife-edge tests the data obtained show a satisfactory tangent to the Euler curve at $L/\rho=90$. Insufficient test data were available, however, to follow the flat end curve down to a well established junction with the Euler curve. The curve drawn, therefore, is not final, but it does not appear to be unreasonable and indicates that the assumption of C=3 for flat end tests is not greatly in error.

The use of previously tested stiffeners may be open to question. Every precaution, however, was used in selecting only undamaged stiffeners.

The curves indicate an Euler-Johnson or similar relationship between failing stress and slenderness ratio. The position of the curve for any other stiffener thickness below 0.029 would change, being lowered, and considerably flattened, judging by the tests on other stiffener sections.

In applying the results of tests on flat ends to a definite structure such as a box section wing, it does not appear unreasonable to apply a correction to flat end test results to bring them to whatever value of fixity is assumed, or determined to exist in the actual structure. If figure 36 were a design column curve, for instance, and it was determined experimentally that a coefficient of 2 was all that could be allowed, a curve midway between those drawn would become the design curve for C=2. The procedure would be similar for C=1, etc. In this respect, published data justifying the use of C=2 or more are few for the usual box wing construction where the sheer webs are widely separated. Values of 1.5 or below are more representative. The coefficient increases somewhat when multiple webs exist, reaching values sometimes in excess of 2. Lacking experimental verification, values in excess of 1.5 appear unconservative. The application of flat end tests directly without a proper fixity allowance is also unconservative.

Since the ratio of stress for C=3 to the stress for C=1 varies from one at $L/\rho=0$ to 3 in the Euler range, the seriousness of flat-end allowables is less in the short column range where the majority of compression members are likely to fall.

Effective radius of gyration

The establishment of the sheet-stiffener units, or the linear relationship between total load and number of stiffeners, and the column curves from the single stiff-

ener tests suggests an investigation to determine the possibility of a relationship between the two.

Accordingly, from table III a 0.029 stiffener series of data was selected for examination, as the values of C computed for that data most nearly agreed with the curve of C finally drawn on figure 26. From the data, the width of sheet working with the stiffener was determined and the radius of gyration of the unit of sheet and stiffener was computed, using the moment of inertia of the unit about an axis through the unit center of gravity and parallel to the sheet.

The computations are tabulated in table VI. It will be noted that although the width of sheet working with the stiffener may vary as the sheet thickness varies, the radius of gyration of the unit remains nearly constant. The computed values of slenderness ratio ranged from 41.5 and 46 for a 9%-inch test specimen for which the failing stress σ , had previously been established, as compared to 46 for a 9%-inch open section stiffener.

Referring to figure 36 it will be noted that for flatend tests, a stress of 115,000 lb./sq.in. is indicated at a slenderness ratio of 43, which value is neatly bracketed by the foregoing values. Thus it is indicated that the column properties of the sheet stiffener units may be determined from a column curve for the individual stiffeners.

The above comparison may be a bit confusing if it is recalled that σ_{\bullet} is based on a stiffener closed by a sheet of zero thickness whereas the plotted curves are for a 0.019 closing sheet. The stress σ_{\bullet} , however, is probably very nearly constant for varying closing strips and was previously assumed so. The curves used in the establishment of σ_{\bullet} , too, were usually nearly flat below 0.020-inch thickness of closing strip. While curves of P versus L or P/A versus L would show a separation due to closing sheet thickness for the individual stiffener tests, a curve of P/A versus L/ρ would probably show little or no influence of closing strip, except possibly for short lengths and the curve for 0.029 stiffeners with 0.019 closing strip may be considered a representative column above.

The nature of the σ_{\bullet} curves of figure 25 and the expression for effective width of sheet indicate that the effective width coefficient C should be applicable to other lengths than the $9\frac{1}{4}$ -inch length of the specimens that determined it.

Constant o.

The assumption of a constant stiffener failing stress for a given stiffener in the presence of varying sheet may be challenged on the basis of the different types of failure that occurred as the sheet thickness varied.

If anything, the assumption is conservative for practical thicknesses. It will be noted from the curves of total load versus N, that for a given N the load carried varies approximately parabolically with skin thickness. If the stiffener failing stress is constant the increase in load carried must be due to an increase in the area of sheet working with the stiffener. Conversely, if the area of sheet working with the stiffener is constant, the failing stress of the stiffener must increase enormously to account for the rapid rise of load with skin thickness. Due to the nature of the stiffeners, and the

susceptibility to local or elastic failures, it is not felt that the latter course is likely to be followed, and that the former is the more likely. It will be granted that there is perhaps a path some where between the extremes and that the stiffener failing stress may increase slightly with increased skin thickness up to the point where failures are governed by the outstanding welding flanges. Since the intermediate path cannot be ascertained, it is felt desirable to proceed with the assumpttion of constant σ_a .

It is likely that in the case of stiffeners rolled or drawn from a single strip, and having no outstanding welding flanges, the greatest variation of σ_a with sheet thickness would occur.

The parabolic relationship between load and skin thickness for a constant stiffener thickness suggests that extrapolating the parabola down to zero skin thickness would be a simpler procedure for determining σ_* in the presence of a sheet of zero thickness than the procedure used previously.

Material

Failing stresses were undoubtedly influenced to some extent by variations in the physical properties of the material, inasmuch as from table I it will be noted that moduli of elasticity varied considerably as did the proportional limit of the material. The extent to which these variables influenced the results is not felt to be large, but it is at the same time indeterminate, as it was not practicable to determine the characteristics of each and every specimen. There could be nearly as great a variation in the properties of the individual elements of the specimens as there is indicated for the material in general, as the stiffeners and sheet could not come from the same stock.

Application to other stiffeners

The results thus far presented have been based on flat specimens employing closed section stiffeners of a particular type.

Neither specimens nor data are available to determine the validity of the application of the results to open section stiffeners attached to the sheet by a single line of welds instead of the two lines used in the test specimens. The effect of curvature was not investigated due to lack of time and facilities for the investigation of the cylindrical specimens.

It appears quite likely that the effective width would decrease somewhat for stiffeners attached by a single line of welds.

Comparison with aluminum alloy sheet-stringer tests

As a matter of curiosity the results of an extensive series of tests on flat ended aluminum alloy plate stringer combinations were examined to ascertain whether or not the linear relationship between load and number of stiffeners applied to the aluminum-alloy specimens. Due to the confidential nature of the data, it cannot be presented herein. Only a few of the results permitted plotting of load against number of stiffeners. The results, however, were such as to verify the linear relationship rather conclusively for the particular stiffener section, sheet thickness, length, and rivet

pitch used. All of the latter variables will undoubtedly effect the total load carried, or the average failing stress, but for particular combinations, it appears that the behavior determined for the stainless-steel specimens applies also to the aluminum-alloy specimens.

The same data afforded a rough comparison of the merits of 24ST aluminum alloy and stainless steel plate stringer combinations: The comparison was made on the basis of the average stress developed in sections of the same pitch, same slenderness ratio of the stiffeners alone, and equal thickness of sheet and stringer, 0.018 for the steel and 0.050 for the aluminum alloy, the ratio of the latter values being closest to the ratios of unit weights of the materials.

Reduced to an L/ρ of 46, corresponding to the 9½-inch A series specimens, and a 6-inch stiffener spacing, it could be expected that average stresses of 67,500 lb./sq. in. and 26,870 lb./sq. in. could be developed for the two materials.

The ratio of these two is 2.51 which falls short of the 2.86 necessary for equal efficiency by about 14 percent, the aluminum alloy making the better showing. The above comparison, however, is not absolute, as the stiffeners were of radically different cross section. The aluminum alloy stiffener sections were of one piece with no outstanding flanges except at the attachment to the sheet. The rivet pitch was ¾ inch. Should the latter figure be increased, a more favorable comparison would be likely to result. Likewise, if the stainless-steel stiffeners were of a cross section similar to the aluminum alloy in freedom from outstanding flanges, a more favorable comparison would be likely.

In general, the stainless-steel combinations do not appear to have any hopeless or unsurmountable disadvantages as compared to aluminum alloy on the basis of structural strength.

Application to box sections

A question will doubtless arise as to whether the design of a box beam for instance should be based on an average stress basis, or on the basis of the σ_{\bullet} stress developed on the most stressed stiffener, and the effective width of sheet working with it.

In order to investigate this point in a simple manner the section shown in figure 4 will be analyzed on the two bases.

Computing the apparent moment of inertia of the section, assuming all sheet effective in compression:

$$\frac{I_{oo}}{2} = 0.010 \times 25.87 \times 7.5^{2} + 6 \times 0.0855 \times (7.5 - 0.333)^{2}$$

$$I_{oo} = 82 \text{ in.}^4$$

The failing stress of the 0.030 stiffener will be 115,000 lb./sq. in.= σ_a .

A mean value of E of 26,000,000 lb./sq. in. will be used.

$$W = C \sqrt{\frac{E}{\sigma_s}} t$$

for $t=0.010 \ C=10$, from figure 26

$$W = 10\sqrt{\frac{26 \times 10^6}{1.15 \times 10^5}} \times 0.010$$

=1.505 in.

$$I_{oo} = (0.010 \times 5 \times 1.505 + 0.875)7.5^2 + 26.4 + 41$$

= 4.74 + 26.4 + 41 = 72.14 in.4

Computing the new C. G. location

$$d = \frac{+0.2587 \times 7.5 - 0.010 \times 8.40 \times 7.5}{0.2587 + 12 \times 0.0855 + 0.0840}$$
$$= \frac{7.5 \ (0.2587 - 0.0840)}{1.3707} = 0.955 \ \text{in}.$$

The effective moment of inertia about an axis through the new C. G. is

$$I_{xx} = I_{oo} - Ah^2$$

= 72.14-1.3707 (0.955)²
= 70.89 in.⁴

On the basis of average P/A the 0.030-0.010-5-inch pitch combination could be expected to develop for C=3 conditions 86,500 lb./sq. in.

The 0.030 stiffener on the individual stringer basis could be expected to carry 115,000 lb./sq. in.

Let us now investigate the moments that could be developed for the two cases:

Section modulus

$$\frac{Y_1}{I_1} = \frac{7.5}{82} = 0.0915 \qquad \frac{Y_2}{I_2} = \frac{8.455}{70.89} = 0.119$$

$$M = \frac{S}{Y} = \frac{86,500}{0.0915} \qquad = \frac{115,000}{0.119}$$

$$M_1 = 945,000 \qquad M_2 = 966,000$$

to cause failing stress.

From the foregoing simple example the difference in moments developed for the trial section on the two bases are so small (2 percent) that it may be concluded that either method would lead to the same result.

The effective EI of the beam, however, would be most nearly represented by the lower of the two EI's for purposes of calculating deflections.

The assumption that the modulus of rupture at failure, in bending, could be computed from the expression $\frac{My}{I}$, which is an almost universal procedure, is verified by test. To obtain consistency with compression machine results on small specimens, however, due account must be taken of length and end fixity.

Variation of stiffener cross section

The EA, EB, and EC series tests were for the purpose of investigating effects, due to varying the stiffener dimensions.

It was originally intended that as the stiffener dimensions varied the length would also be varied to maintain a constant slenderness ratio based on the properties of the stiffener alone, that being the only possible variable that was contemplated at the time. The slenderness ratios desired were 25, 50, and 75. In the process of adding welding flanges to the sections finally used, exact control of the length was lost with the result that the slenderness ratios as computed for the final sections and the lengths furnished were of the order of 23.2–24.8, 46.4–49.6, and 69.5–74.4 for the series.

EA series

This series of tests was intended to supplement the A series tests by determining the influence of variations of the stiffener dimensions. The specimens were originally chosen to have a constant slenderness ratio, based on stiffeners alone. Accordingly, as the stiffener depth increased, the specimen length increased.

It has been shown in the preceding discussion that the governing slenderness ratio is not that involving the ρ of the stiffener alone, but is that determined by the sheet that is working with a stiffener. Accordingly, the original selection of constant stiffener L/ρ was not entirely satisfactory for the purpose intended.

For the 0.009 stiffener EA specimens a decrease in average failing stress is noted as the stiffener depth increases. This may logically be charged to decreasing stability of the stiffeners as the depth increases. The influence of slenderness ratio and sheet thickness are not pronounced for these thicknesses.

The 0.029 stiffener EA specimens show a much higher average failing stress than the 0.009 specimens, an appreciable influence due to slenderness ratio, but little influence due to variation of the sheet thickness. The behavior of the 0.049 stiffener EA specimens was similar to that of the 0.029 with a generally slightly higher average stress developed. In these tests, again it was evident that the stiffener is primarily responsible for the strength properties of the specimens.

The 0.029 stiffener EA specimens indicate a tendency toward a maximum average stress for a stiffener of %-inch depth, whereas the 0.049 stiffeners indicate a possibility of a maximum average stress for stiffeners of about %- to 1-inch depth for a %-inch width.

The maximum stresses appear, for a given stiffener, to decrease somewhat as the sheet thickness increases. With all thicknesses of stiffeners, it was noted during tests of the EA specimens that at depths of 1 inch or more the predominating type of stiffener failure was by lateral buckling of the stiffener as a whole rather than a collapse of its component parts.

EB series

The 0.009 stiffener EB specimens show little variation of average failing stress for any of the variables entering. For the 0.029 stiffener specimens there is apparent a decrease in average failing stress with increasing stiffener widths, the decrease, however, is greatest for the 0.009 skin and practically disappears for the 0.049 skin. Length appears to have an appreciable effect.

For the 0.049 stiffeners the same behavior was noted as for the 0.029 stiffeners, with the length effect increasingly apparent.

EC series

It would appear from an examination of the EC series of data that the effect of increasing both the stiffener width and depth might be expressed as the summation of the effects of increasing first the depth and secondly the width.

The data indicate that the slope of the curves of the EC data for 0.009 stiffeners are approximately the sums of the slopes of corresponding curves of the EA

and EB data. For the higher stiffener thicknesses the average failing stresses correspond closely with those of the EA series, and the influences of the independent variables appear the same.

Open section stiffener tests

The results of flat and column tests on stiffeners typical of the A, B, and C series stiffeners are shown in figure 69.

The open section tests indicate an appreciable increase in failing stress with increase in thickness and do not exhibit the tendency of constant failing P/A for thicknesses in excess of 0.029 determined from the closed section specimen tests.

The failures for the longer lengths were predominantly due to twisting. The results do not appear at all useful in predicting the strength properties of platestringer combinations and are presented only as data on particular open sections.

Formation of wrinkles

It was noted during the tests that on the lighter skins the formation of wrinkles started almost at no load, and was influenced by the width or spacing of weld lines. The wrinkle pattern was generally such that pitch of the wrinkles was equal to the distance between weld lines. In other words, the wrinkle pattern formed a series of squares.

Wrinkling loads were practically always lower by considerable amount than failing loads. In the case of some of the heavier gages of the EA, EB, and EC series stiffeners, another type of wave was observed, involving the reinforcing groove at the side or back of the stiffener. In this case, the formation of waves only slightly preceded failure, and was largely responsible for it. In this case, the pitch of the waves was 3 to 4 times the width of the stiffener side. It was this type of waving that usually precipitated explosionlike failures in which the sections burst as if blown apart over a considerable length. The waves undoubtedly imposed very severe loads on the welds, and the welds were likely unable to hold the elements in place at the advanced stages of waving. See figure no. 76.

Dimensions of outstanding legs

Nominal t	ь	b/2t
0. 010 . 015 . 020 . 030 . 050	0. 187 . 187 . 187 . 187 . 187 . 250	6. 35 4. 23 3. 18 2. 12 2. 50

Dimensions of flat across A section stiffeners

Nominal t	W	0.5/t
0. 010 . 015 . 020 . 030 . 050	0. 5 . 5 . 5 . 5	50 33. 3 25 16. 7 10

Types of failure

The type of failure showed a general classification into several fairly well defined groups. These are listed and described briefly as follows:

Type A.—A failure due to the buckling of the outstanding legs of the stiffener. It was confined usually to the thinner stiffeners. The appearance of the failures is well illustrated at the top and bottom of the right-hand specimen of figure no. 83, and by figure no. 80.

Type B.—When the combined thicknesses of the stiffener and sheet were less than twice the stiffener thickness, the failures were generally precipitated by buckling of the stiffener welding flanges. This type of failure was common to the specimens having very thin sheet. The appearance of this type of failure is shown in figure no. 81.

Type C.—When the stiffener dimensions became sufficient, the sides or backs of the stiffeners became unstable, permitting the formation of waves in those elements which led to their collapse. This type of failure is shown in figures nos. 82 and 90.

Type D.—When a heavy sheet was used in the presence of light stiffeners, a failure such as shown in figure no. 83 occurred. In this figure it may be noted how the heavy sheet has bowed toward the stiffeners, and how the outstanding legs of the stiffener at the failed section have bowed simultaneously toward the sheet.

Type E.—When extreme differences between sheet and stiffener thickness existed, failure often occurred due to the pulling of slugs of welded material from the lighter sheet by the heavier, as may be seen in figure no. 84. This failure is not due to faulty welding but due to too few, or too small welds, and is a somewhat abnormal condition.

Type F.—Due to the formation of severe buckles in the sheet, failure of stiffeners sometimes occurred due to the distortion of the stiffener welding flanges by the sheet buckling. The appearance of the failed specimens was practically the same as that shown in figure no. 85.

Type G.—This failure was due to column failure of the specimens, and was noted before complete collapse usually in the heavier stiffener sections. This type of failure is shown in figures nos. 86 and 90.

Type H.—When there was a rough balance of the elastic stability of all of the elements of the specimens the resulting failure was a general collapse of the complete section. This is illustrated by figure no. 87.

Type 1.—When, as in the EA series, the ratio of depth to width of the stiffener exceeded about 1.5 the failure was due to lateral instability of the stiffeners, and was characterized by a waving of the stiffener in a direction parallel to the sheet. Figure no. 88 shows a typical failure of this type.

Type J.—This classification includes all failures due to poor welds and weld failures, due usually to the lack of fusion of the welded material. The greatest portion of these failures occurred when thicknesses of 0.030 and 0.050 were encountered. The poor spot welds and complete disintegration, due to poor welds, will be noted in figure no. 89. Figure no, 90 shows a side view of the failure.

fied, and typical buckling failures are shown in figures no. 91 (corrugated sheet alone) and no. 93 (corrugated sheet with flat sheet attached).

Typical column failure for the corrugated sheet is shown in figure no. 92 and for the corrugated sheet with flat sheet attached in figure no. 94.

Weld failures

In the EA, EB, and EC series, particularly for the thicker sheets, considerable difficulty due to poor welding was encountered. Due to the sudden nature of the failures in the majority of cases, it was not always possible to detect weld failures prior to a general exploding of the specimen.

For this reason, it is believed that the scattering of many of the test points is due to weld failures that were not detected. In cases where welds failed prior to a general failure of the specimen, the test was discontinued and a gap in the data exists.

It was rare that a failed weld showed evidence of pulling a slug of welded material from the thinner of two sheets joined together. The majority of the failed welds showed a very small fused area in comparison to the electrode diameter. External appearances of the welds gave no indication of the condition of the welds. In some instances it was noted after failure that attempts had been made to weld through a layer of paint applied along a line of welds due to a specimen number painted on a sheet. Also it was noted on a number of specimens that the specimens had been rewelded over the original welds.

The spot-welding ability or technique of the contractor is held responsible for the failures; and spot welding of stainless steel, as a whole, should not be condemned due to the unsatisfactory nature of the welds experienced during these tests.

Judging from tests on specimens procured from the same and other sources, it is quite possible to so weld the specimens used for these tests that no welds would have failed in sheets of similar thicknesses no matter what the degree of crushing of the specimens might have been.

Since the initiation of the project, Air Corps Specification No. 20011, dated January 27, 1934, has been issued. The essential items of the specification are the requirements of thyratron-tube control and the establishment of standard weld strengths for specific sheet thicknesses to be determined by tension tests on strips joined by a single weld.

The use of a thyratron control is excellent but not essential, and it might be mentioned in passing that the contractor used a thyratron control for timing the welds, and during the later stages of fabrication checked welds for consistency by tension tests and still the welds failed under compressive loads.

Accordingly, it is believed that specification no. 20011 is inadequate in itself in guaranteeing freedom from weld failures on members subject to compression and that it should be amended, or procurement specifications or contracts so worded as to require that in addition to compliance to specification no. 20011 a contractor must demonstrate by suitable compression | 0.01 inch of covering thickness up to a thickness of

The failures of the corrugated sheets were not classi- | tests that welded, built-up sections of his design are capable of withstanding crushing after reaching their failing load without weld failures.

I series corrugations

It was hoped that a single curve for the ultimate compressive or buckling stress for the corrugated sections could be established in terms of R/t, R being the radius of curvature of the corrugations and t the thickness. It was found that R/t and L/ρ , or slenderness ratio, were interdependent and that column curves could be established for certain R/t's and buckling curves for certain L/ρ ratios, but that no one curve would express an upper limit.

It will be noted that the curves for certain R/t values have been drawn tangent to a Euler curve for C=3, in figure 64.

Previous tests on aluminum-alloy corrugations had indicated that flat-end test conditions closely approximated the end conditions for a fixity coefficient of 3.

In table VII a comparison is made between the strength properties of stainless steel corrugated sheet 24ST aluminum alloy corrugated sheet. The comparison is made on the basis of equal weights of cross section

for identical slenderness ratios of 35, 70, and 100.

The ratio of weights is $\frac{0.286}{101}$ or 2.83. In selecting stresses for stainless steel, the R/t has been increased by multiplying by 2.83, corresponding to an equivalent reduction in thickness.

For sheets of aluminum alloy or stainless steel to carry equal loads, the stress developed in the stainless steel must be 2.83 times that developed in the aluminum alloy. Hence, in table VII whenever the ratio of stresses exceeds 2.83 steel offers an advantage in strength over 24ST aluminum alloy sheet of equal weight.

In selecting sections on the basis of equal strength, a more proper basis, the weight ratio would not be quite as large as the strength ratio on the basis of equal weight due to the effect of reducing the thickness of the steel somewhat to bring the strength ratio to one.

In the foregoing table no account has been taken of the presence of any covering skin, inasmuch as for the purposes of this discussion, covering skins of proportional thicknesses are in order. One item that will operate against aluminum alloys in comparing with stainless steel is that of protective coatings, which may reach as much as 10 percent of the aluminum alloy weight for thin sheets.

In order to investigate the effect of covering skin on the average stress developed in covered corrugations, flat skin of varying thickness was welded to the corrugations and the resulting specimens loaded in compression.

It was noted throughout these tests that the flat covering skin became wrinkled at very low loads, and that the wrinkle pitch was, on an average, close to the pitch of the corrugations, regardless of the cover skin thickness.

It will be noted from figure 66 that the average failing stress drops off at a rate of about 20,000 lb./sq. in. per about 0.020 inch for 1-inch pitch corrugations, after which the average stress begins to rise again. It is unfortunate that the 0.029-inch thickness was not great enough to determine how far this rise might be carried.

In figure 68 failing loads have been plotted against covering thickness. It will be noted that the total loads vary in a peculiar manner, increasing as the covering thickness increases, in spite of a lowering of the average failing stress.

The data are not as consistent as might be desired, due to the fact that this series of tests was performed by inexperienced personnel, and were the very first tests conducted during the investigation.

Suggestions for future work

Insufficient time has been available to fully investigate all the points suggested during the examination of the available data. The following topics are suggested to anyone interested in a continuance of the investigations, or who may be engaged in a similar investigation:

- 1. The influence of stiffener sections of other shapes, notably sections having as outstanding legs only the flanges necessary to weld it to the sheet.
- 2. The development of a most generally efficient stiffener section.
- 3. An investigation of the influence of stiffeners attached by a single row of welds on the effective width of sheet acting with the stiffener.
- 4. An investigation of the possibility of determining effective width of sheet in the case of riveted sheet stiffener combinations, particularly in aluminum alloy.
- 5. An investigation to determine the effect of curvature on the effective width of sheet working with the stiffener.

NOTE

Since the writing of this report, completed about June 1, 1934, those specimens that failed, due to weld failures, have been replaced by the vendor.

The results of tests on these replacement specimens will be presented in an appendix to this report as soon as the data become available, together with data on sheet-stringer combinations involving stiffeners of sections differing from those used in the main investigation.

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- 7. Matériel Division, E. S. M. R., Serial No. Str-51-26 (Addendum 2), Compressive Tests on Miscellaneous Stainless Steel Sections for P-26 Stainless Steel Wings by S. R. Carpenter and C. G. Brown.
- 8. M. I. T. Paper, Notes on the Design of Metal Parts for Use in Aircraft Construction.

Table I.—Physical properties of test specimen material
[Material Branch tests, reference series R34-93]

Specifi-			Proport	ion limit	Yield point	Elongation	Modulus
cation	Thickness	U. T. S.	Tangent	0.0001 inch per inch	(0.002 inch per inch)	(percent in 2 inches)	elasticity (1,000 lb./ sq. in.)
1 1 2 2 3 4 5 6	0.0000 .0090 .0090 .0094 .0148 .0147 .0142 .0144 .020 .020 .020 .0290 .0307 .0288 .0494 .0473 .0490	176, 980 191, 840 183, 300 188, 800 187, 700 188, 800 179, 400 191, 400 180, 500 185, 200 175, 800 177, 800 182, 600 171, 700 180, 160	55, 440 60, 990 54, 240 53, 080 57, 350 54, 350 84, 390 83, 200 55, 550 48, 480 50, 630 48, 880	72, 100 94, 300 67, 700 87, 500 77, 550 92, 800 107, 490 	163, 000 157, 460 168, 150 144, 400 157, 300 154, 210 164, 300 164, 300 162, 150 148, 690 157, 800 161, 700	3.55 7.50 8.55 6.55 5.05 5.05 5.05 5.05 5.05 5.05 5	26, 720 27, 780 27, 780 29, 850 29, 020 31, 030 27, 430 28, 700 25, 200 25, 220 24, 840 28, 450

Table II.—Computation of stiffener failing stress LENGTH, 9.25 INCHES

0.009 STIFFENER

Sheet thickness (inch)	Sheet area (inch)	Stiffener area (inch)	Total area (1 stiffener)	Failing load (5 stiffeners)	Stiffener failing stress (lb./sq. in.)	σ,
0.009 .014 .018 .028 .048	0. 0079 . 0122 . 0158 . 0245 . 0420	0. 0286 . 0286 . 0286 . 0286 . 0286	0. 0365 . 0408 . 0444 . 0531 . 0706	14, 000 16, 000 20, 000 27, 700 38, 000	76, 700 78, 300 90, 200 104, 300 107, 600	75, 000
'		0.0	14 STIFFEN	ER		
0.009 .014 .018 .029	0. 0079 . 0122 . 0158 . 0254	0. 0424 . 0424 . 0424 . 0424	0. 0503 . 0546 . 0582 . 0678	24, 300 28, 000 31, 400 41, 000 54, 000	96, 600 102, 200 108, 000 121, 000 128, 000	90, 000

0.019 STIFFENER

	i	I	ī .	-		1
0.009	0.0079 .0122	0. 0548 . 0548	0. 0627 . 0670	35, 000 39, 300	111, 800 117, 200	
. 018	. 0158	. 0548	. 0706	42, 200	120, 000	115, 000
. 029 . 048	. 0254	. 0548	. 0802	54, 000 73, 500	134, 000 152, 000	il

0.029 STIFFENER

0. 009	0.0079	0. 0809	0. 0888	52,000	117,000
. 014	, 0122	. 0809	. 0931	54, 200	116, 400
. 018	. 0162	.0809	. 0971	58, 300	120,000
. 029	. 0254	. 0809	. 1063	70,000	131,000
. 048	. 0420	. 0809	. 1329	95, 000	136, 6

0.049 STIFFENER

	0.009 .014 .018 .029 .048	0.009 .014 .018 .029 .048	0. 144 . 144 . 144 . 144	0. 153 . 158 . 162 . 173 . 192	88, 000 91, 000 95, 000 112, 500 145, 500	115, 000 115, 000 117, 200 130, 000 151, 500	115, 000
- 1		Į.			1		

LENGTH, 3 INCHES

		DENGIL	, 0 1110112		
Sheet thickness (inch)	Stiffener thickness (inch)	Total area (1 stiff- ener)	Failing load per stiffener	Stiffener failing stress (lb./sq.in.)	σ,
0.009 .019 .029	0.019	0.0627 .0716 .0802	8, 030 9, 000 11, 200	127, 800 125, 600 139, 500	125,000
. 009 . 019 . 029	. 029	.0888 .0977 .1063	13, 140 14, 380 16, 200	148, 100 147, 000 152, 400	145, 000
	<u> </u>	LENGTH,	6 INCHES		
0.009 .019 .029	0.019	0.0627 .0716 .0802	7, 750 8, 700 10, 930	123, 400 121, 300 136, 000	120,000
.009 .019 .029	. 029	. 0888 . 0977 . 1053	11, 800 13, 080 15, 460	133,000 134,000 145,100	131,000
		LENGTH,	12 INCHE	S	
0.009 .019 .029	0.019	0.0627 .0716 .0802	6, 450 7, 620 9, 400	102, 800 106, 400 117, 200	102, 500
. 009 . 019 . 029	.029	. 0888 . 0977 . 1063	9, 170 10, 470 12, 910	103, 200 107, 200 121, 500	103, 000
		LENGTH,	18 INCHE	S	
0.009 .019 .029	0.019	0.0627 .0716 .0802	4,650 5,750 7,000	74, 100 80, 300 87, 200	71, 500
. 009 . 019 . 029	029	. 0888 . 0977 . 1063	6, 530 7, 900 8, 550	73.500 75,700 80,200	73, 200

Table III.—Computation of effective width of sheet working with stiffeners

A SERIES TESTS N=5 Stiffener t=0.009 $\sigma_{\bullet}=73,000$ $\sqrt{\frac{E}{\sigma_{\bullet}}}=18.88$

t	P	$\frac{P}{(N-1)\sigma_{\bullet}}$	$\frac{N}{N-1}A_{ii}$	$\frac{0.875t}{N-1}$	A.h	w	$\frac{W}{t}$	C
0.009 .014 .018 .028 .048	14,000 16,000 19,000 28,000 38,000	0. 0479 . 0548 . 0651 . 0959 . 1300	0. 0358	0.0020 .0031 .0040 .0063 .0108	0. 0101 . 0159 . 0253 . 0538 . 0834	1. 121 1. 137 1. 405 1. 920 1. 735	124, 5 81 78 68, 5 36, 1	6. 6 4. 3 4. 1 3. 6 1. 9
···········	`	Si	Eiffener $t = 0.014$	$\sigma_s = 90,000$	$\sqrt{\frac{E}{\sigma_i}}$ =17.0	0		
0.009 .014 .018 .029 .048	24, 500 28, 000 31, 400 41, 000 54, 000	0. 0675 . 0778 . 0872 . 1138 . 1500	0.0530	0. 0020 . 0031 . 0040 . 0063 . 0108	0. 0225 . 0217 . 0302 . 0545 . 0862	2. 50 1. 55 1. 68 1. 88 1. 79	278 111 93 65 37. 5	16. 4 6. 5 5. 4 3. 8 2. 2
		Si	iffener $t=0.019$	$\sigma_{i} = 105,00$	$\sqrt{\frac{E}{\sigma_e}}=15.$	75		
0.009 .014 .018 .029 .048	35, 000 39, 300 42, 200 54, 000 73, 500	0. 0834 . 0937 . 1005 . 1285 . 1750	0.0675	0.0020 .0031 .0040 .0063 .0108	0. 0139 . 0231 . 0290 . 0547 . 1067	1. 55 1. 65 1. 61 1. 88 2. 27	171 118 90 65 47. 3	10. 8 7. 5 5. 7 4. 1 3. 0
		St	iffener $t=0.049$	$\sigma_{s} = 115,00$	$\sqrt{\frac{E}{\sigma_s}} = 15.$	02		
0.009 .014 .018 .029 .048	87, 500 92, 500 96, 000 110, 500 145, 500	0. 1900 . 2010 . 2085 . 2400 . 3180	0. 180	0. 0022 . 0035 . 0045 . 0073 . 0120	0. 0078 . 0175 . 0245 . 0527 . 1260	0. 866 1. 245 1. 36 1. 82 2. 62	96. 5 89 75. 5 62. 6 54. 6	6. 4 5. 9 5. 0 4. 1 3. 6

B SERIES TESTS

$N=4 \text{Stiffener } t=0.009 \sigma_{\star}=73,000 \sqrt{\frac{E}{\sigma_{\star}}}=18.88$ $\begin{array}{c ccccccccccccccccccccccccccccccccccc$				2 01					
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$			N=	4 Stiffener t:	=0.009 σ,=	=73,000 $\sqrt{\frac{E}{\sigma_*}}$	=18.88		
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$. 020 . 030 . 040	16, 500 21, 750	. 0753 . 0993 . 1238	0. 0515	. 0058 . 0087 . 0117	. 0391 . 0606	1. 303 1. 517	43. 5 37. 8	2. 3 2. 3 2. 0 1. 7
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$,			Stiffener $t=0.0$	19 σ _e =105,	$000 \sqrt{\frac{E}{\sigma_s}} = 1$	5. 75		
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$. 020 . 030 . 040	35, 750 43, 500 52, 700	. 1134 . 1428 . 1728	0.0730	. 0058 . 0087 . 0117	. 0346 . 0568 . 0881	1.73 1.89 2.20	86 63 55	10. 3 5. 4 4. 0 3. 4 2. 9
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$			St	iffener $t=0.029$	θ σ.=115,00	$\sqrt{\frac{E}{\sigma_s}} = 15.$	02		
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$. 020 . 030 . 040	50, 000 59, 000	. 1450 . 1680 . 1984	0. 1078	. 0058 . 0087 . 1168	. 0314 . 0515 . 0790	1. 57 1. 72 1. 98	78 57 49	10. 3 5. 1 3. 7 3. 2 2. 7
0.010 70,000 0.2030 0.192 0.0033 0.008 0.80 80 020 76,500 2220 0.066 0.009 0.46 1.53 51			Si	iffener $t=0.049$	$\sigma_s = 115,00$	$\sqrt{\frac{E}{\sigma_s}}=15.$	02		
. 020	t	P	$\frac{P}{(N-1)\sigma_t}$	$\frac{N}{N-1}A_{ii}$	$\frac{1.00t}{N-1}$	Ash	W	$\frac{W}{t}$	C
.000 .11/ 2.34 4/	. 020 . 030	76, 500 85, 500	. 2220	0.192	.0066	. 046	1, 53	51	3. 3 ·3. 2 3. 1

Table III.—Computation of effective width of sheet working with stiffeners

C SERIES TESTS

		N=5 St	iffener $t = 0.01$	9 L=6 σ,	=120,000 🗸	$\frac{E}{\sigma_0} = 14.71$		
ı	P	W	11'	C				
0.009 .019 .029	38, 750 43, 500 54, 800	0. 0807 . 0906 . 1141	0.0675	0.0020 .0042 .0065	0. 0112 . 0189 . 0401	1, 248 , 995 1, 384	138 52. 4 47. 7	9. 30 3. 51 3. 23
			$L=12$ σ_{\bullet}	=102,500	$\sqrt{\frac{E}{\sigma}} = 15.90$			
0.009 .019 .029	32,000 38,000 47,000	0. 0780 . 0926 . 1145	0, 0675	0.0020 .0042 .0065	0. 0085 . 0209 . 0405	0. 945 1. 10 1. 395	105 58 48	6. 6 3. 6 3. 0
,			L=18	$\sigma_i = 71,500$	$\sqrt{\frac{E}{\sigma_s}} = 19.08$			
0.009 ;019 .029	23, 100 28, 750 35, 000	0.0808 .1005 .1224	0. 0675	0.0020 .0042 .0065	0. 0113 . 0288 . 0504	1, 255 1, 514 1, 735	139, 5 79, 6 59, 8	7. 3 4. 1 3. 1

Table IV.—Column tests—individual stiffeners

[Stiffener thickness, 0.029 inch]

FLAT-END TESTS

Length (inches)	Sheet thickness (inch)	ρ (inch)	L/ ho	Failing load (pounds)	Area ! (square inch)	Failing stress (lb., sq. in.)
0. 97 . 94 . 89 1. 46 6. 00 9. 23 9. 23 9. 22 2. 20 13. 81 13. 84 13. 82 18. 17	0, 021 . 021 . 019 . 021 . 021 . 019 . 019 . 019 . 019 . 021 . 019	0. 2247 . 2247 . 2235 . 2247 . 2247 . 2236 . 2235 . 2235 . 2235 . 2247 . 2235 . 2247 . 2237 . 2247	4. 3 4. 2 4. 0 6. 5 26. 7 41. 3 41. 3 41. 2 41. 1 61. 5 62. 0 61. 5 82. 6	14, 320 14, 010 13, 845 13, 800 13, 400 10, 860 10, 150 11, 000 11, 620 9, 040 9, 440 9, 170 8, 480	0. 1045 . 0965 . 1022 . 1032 . 1002 . 0946 . 0960 . 0956 . 0946 . 0985 . 0344 . 0988 . 0970	137, 000 145, 300 135, 400 133, 800 133, 700 105, 800 114, 700 102, 800 91, 700 100, 000 92, 900 87, 400
	<u> </u>	KNI	FE-EDGE	TESTS 2		
3. 48 4. 49 7. 54 7. 54 10. 72 10. 70 15. 32 15. 34 19. 97	0. 019 . 019 . 020 . 021 . 019 . 019 . 021 . 019 . 019	0. 2235 . 2235 . 2240 . 2247 . 2235 . 2235 . 2247 . 2235 . 2235	15. 6 20. 1 33. 6 33. 5 48. 0 47. 9 68. 1 68. 6 89. 4	12, 485 12, 080 9, 470 10, 240 6, 800 7, 500 4, 630 4, 870 2, 765	0. 0954 . 0959 . 0972 . 0994 . 0956 . 0946 . 0988 . 0944	131, 000 126, 000 97, 500 103, 200 71, 100 79, 300 46, 800 51, 600 28, 500

¹ Areas were determined by weighing specimens.
² In the knife-edge tests, L includes 1.50 inches due to the knife-edges.

Table V.—Section properties of stiffeners 1/2×1/2×0.030 CLOSED SECTION STIFFENERS

Closing strip thickness (inch)	Total area (square inch)	C. G. loca- tion (inch)	Ic. g. (inch) 4	ρ (inch)
0. 010	0. 0943	0. 301	0. 00437	0. 216
. 020	. 1030	. 274	. 00518	. 224
. 030	. 1118	. 251	. 00587	. 229
. 050	. 1293	. 211	. 00721	. 236

1/2×1/2 OPEN SECTION STIFFENERS

ffener t inch)	Area (square) inch)	C. G. loca- tion (inch)	Ic. c. (in.)4	ρ (inch)	A/t (inch)
0. 010 . 020 . 030 . 050	0. 0286 . 0572 . 0855 . 1429	0. 333 . 333 . 333 . 333	0. 00115 . 00230 . 00343 . 00575	0. 2005 . 2005 . 2005 . 2005	2. 8575 2. 8575 2. 8575 2. 8575 2. 8575

Table VI.—Computation of slenderness ratio of working units of sheet and stiffener

	Stiffener			8	Sheet !		Total	Total	d'= 0.333A.++			ρ=			
ı	Area	Ib	t	B.	Area	Ib	area	Ib	$\frac{\frac{t}{2}A_{th}}{A_{T}}$	$A_i(d')^2$	Ic.g.	$\sqrt{\frac{I}{A}}$	L	L/ ho	σ,
0. 0283	0. 0809	0. 01220	0, 010 . 020 . 030 . 040 . 050	1. 54 1. 57 1. 72 1. 98 2. 11	0. 0154 . 0314 . 0515 . 0790 . 1057	0. 0000005 . 0000042 . 0000155 . 0000423 . 000088	0. 0963 . 1123 . 1325 . 1601 . 1864	0. 01220 . 01220 . 01221 . 01224 . 01229	0. 2805 . 2420 . 2190 . 1780 . 1586	0. 00760 . 00660 . 00635 . 00509 . 00470	0. 0046 . 0056 . 00586 . 00715 . 00759	0. 2185 . 222 . 2225 . 211 . 2015	9. 25 9. 25 9. 25 9. 25 9. 25 9. 25	41.5	115,000

From table IV.

Table VII.—Comparison of strength properties of corrugated sheets of 24ST aluminum alloy and stainless steel, for flat end conditions

T.	۱.	_	3	1

Stainl	ess steel	Dι	ıral	Stress
R/l	Failing P/A	R/t	Failing P/A	ratio
33 88 100 162 264	174,000 145,000 132,000 96,000 46,000	10. 5 30. 7 35 57 92	48, 600 33, 200 30, 700 21, 400 13, 200	3. 55 4. 37 4. 30 4. 50 3. 48
	·	$L/\rho = 70$		
33 88 100 163 264	122,000 111,000 106,000 63,000 43,000	10. 5 30. 7 35 57 92	36, 500 33, 200 30, 700 21, 400 13, 200	3. 34 3. 34 3. 46 2. 94 3. 26
		$L/\rho = 100$		
33 88 100 162 264	87, 000 86, 000 84, 000 52, 000 40, 000	10. 5 30. 7 35 57 92	27, 500 27, 500 27, 500 27, 500 21, 400 13, 200	3. 16 3. 12 3. 05 2. 43 3. 02

TABLE VIII .- Stainless steel test specimens

				Failing	Thic	kness			Num-			Percent		
Specifica- tion No.	Weight (lbs.)	Area (square inch)	Failing load (lbs.)	stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	ber of stiff- eners	Area of sheet	Area of stiff- eners	rein- force- ment	Pitch (inches)	Type of fail- ure
A-1a A-1a A-1a A-1a A-1a A-2a A-2a A-2a A-3b A-3b A-4a A-5b A-6a A-7a A-7a A-8a A-9b A-10b A-11b A-12a A-10b A-11b A-12a A-10b A-11b A-12b A-11b A-12b A-11b A-12b A-13a A-14a A-14a A-14a A-14a A-14a A-14a A-15b A-16b A-17b A-18a A-19a A-2b A-2b A-2b A-2b A-2b A-2b A-2b A-2b	0.587	0. 2200	13, 720 14, 470 11, 630 11, 150 9, 700 8, 700 5, 370 5, 870 5, 870 15, 870 16, 18, 120 18, 210 18, 210 18, 210 18, 210 18, 210 25, 730 18, 570 11, 230 25, 730 18, 570 12, 030 7, 400 37, 150 31, 230 35, 900 25, 730 11, 260 23, 600 24, 510 20, 700 10, 180 11, 260	62, 400 67, 250 67, 250 55, 750 55, 200 55, 200 66, 200 66, 200 66, 200 66, 200 66, 200 66, 200 66, 200 66, 200 66, 200 66, 200 67, 600 67, 600 67, 600 67, 600 67, 600 67, 600 67, 600 67, 600 67, 600 68, 200 58, 400 59, 500 51, 100 51, 100 52, 600 52, 600 53, 800 52, 600 60, 80	0.010 0.010	0.009 .009 .009 .009 .009 .009 .009 .00	\$\\\\ 28.25.25.25.25.25.25.25.25.25.25.25.25.25.	8. 33 8. 32 9. 28 9. 28 9. 28 9. 28 9. 28 10. 88 10. 88 10. 88 10. 88 10. 88 10. 88 10. 88 10. 88 10. 88 10. 89 10. 19 10. 10 10. 1	554433000007766443300077554433000775544800775533300054433300077554433333775544330006644333	0.0750 0.7750 0.7750 0.7750 0.7835 0.8331 0.8335 0.6666 1.0866 1.	0. 1450	62. 7 2 9 9 6 6 8 8 6 6 8 8 6 6 8 6	1.86 1.86 1.86 1.86 1.16 1.16 1.13 1.13 1.13 1.13 1.13 1.1	FEFFUHFFFF AAFFHHFFFFFHAOOAAEEEEAEEAEEDEEJFFFFFFFFFFFFFFFFFFFFFFFF

Table VIII .- Stainless steel test specimens-Continued

010		Area	Failing	Failing	Thic	kness			Num-		Area of	Percent		Туре
Specifica- tion No.	Weight (lbs.)	(square inch)	load (lbs.)	stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	ber of stiff- eners	Area of sheet	stiff- eners	rein- force- ment	Pitch (inches)	of fail- ure
A-48b A-49a A-49a A-49a A-49a A-49a A-49a A-49a A-50a A-50a A-51a A-51a A-51a A-51a A-52a A-52a A-52a A-52a A-52a A-52a A-52a A-55a A-56a A-57b A-60a A-60a A-60a A-60a A-60a A-60a A-63a A-73a A-74a A-75a A-75a A-75a A-76b A-78a A-79a A-79a A-80a	2. 258 1. 217 1. 222 1. 2984 2. 886 2. 877 2. 720 2. 509 1. 223 1. 2920 2. 762 2. 762 2. 671 1. 365 2. 671 1. 365 2. 671 1. 365 2. 881	0. 859	19, 600 48, 150 50, 600 36, 800 35, 800 28, 290 21, 900 21, 200 21, 200 23, 300 21, 200 23, 300 21, 200 23, 500 15, 250 45, 300 51, 690 25, 690 61, 69	22, 800 104, 400 109, 300 99, 000 96, 600 62, 400 62, 400 67, 100 74, 500 77, 100 74, 500 92, 200 92, 200 92, 200 93, 450 88, 300 66, 000 66, 000 68, 300 76, 500 98, 600 77, 500 101, 200 78, 500 98, 600 77, 500 101, 200 116, 300 113, 400 118, 300 121, 300 121, 300 1113, 400 121, 300 113, 400 131, 40	0. 015 0.020 0.20 0.20 0.20 0.20 0.20 0.20 0.	0.048 0.099 0.009 0.009 0.010 0.010 0.009 0.014 0.014 0.014 0.014 0.014 0.018 0.018 0.018 0.018 0.018 0.018 0.019 0.029 0.048 0.048 0.048	9. 223 9. 223 9. 224 9. 225 9. 224 9. 225 9. 224 9. 225 9. 224 9. 225 9. 225	9. 30 9. 30 10. 55 10. 55 10. 55 10. 55 10. 21 11. 10 11. 10	27755443322664433332266443322228866443329966443322554433222553332266443333228866443333999775544	0. 7570 0.8834 0.837 0.950 0.944 1218 1210 1090 1090 0.8837 1338 12244 1244 1246 1803 1811 1430 1432 1432 1432 1432 1432 1432 1432 1433 1339 1375 2775 2775 2775 2775 2775 2775 2775 2	0. 1020 37711 3793 2765 2760 2132 21115 1645 1635 1097 1083 33279 3322 22226 2231 1644 1644 1667 1679 1110 1108 3323 3261 3213 3298 1660 1121 1136 1156 11165 1156 11165 11165 11165 11165 11165 11166	66. 0 81. 2 72. 0 81. 2 72. 0 81. 2 72. 0 81. 2 72. 0 81. 2 72. 0 81. 2 82. 0 83. 0 84. 0 85. 0 86. 0	14. 90 1. 40 2. 40 2. 40 2. 40 3. 73 3. 60 8. 40 1. 711 2. 67 2. 67 4. 00 6. 00 6. 00 6. 00 6. 00 6. 00 6. 00 6. 00 6. 54 1. 20 1. 24 1. 24 1. 24 1. 24 1. 24 1. 24 1. 25 1. 26 1. 27 1. 26 1. 27 1. 27 1. 27 1. 27 1. 28 1. 20 1. 20 2. 50 2. 50 3. 60 3. 60	DEEEELEEEEE LEEEEE LEEFEEELLUHUUHHHHHHLILEELLEEGOOOOOLLEELLOEFEEEEEEEEEELEEEEE LEEEEEELEEEEE

Table VIII.—Stainless steel test specimens—Continued

***************************************				Failing	Thic	kness			Num-			Percent		
Specifica- tion No.	Weight (lbs.)	Area (square inch)	Failing load (lbs.)	stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	ber of stiff- eners	Area of sheet	Area of stiff- eners	rein- force- ment	Pitch (inches)	Type of fail ure
A-97a	2. 502	0. 947	48, 800	51, 500	0. 030	0, 048	9. 23	14. 35	3	0. 6890	0. 2580	21. 0	6. 74	J
A-97b	2. 502	. 946	51, 100	54,000	. 030	. 048	9. 25	14. 37	3 7	. 6900	. 2560	20.9	6. 74	J
A-98a	2. 958	1. 115	124, 600	111, 700	. 050	. 010	9. 27	10.35	7	. 1035	1.0115	90. 2	1.56	G
A-98b	2, 906	1.100	119, 200	108, 300	. 050	. 010	9. 24	10.40	7	. 1040	. 9960	90. 1	1. 56	1
A-99a	1.803	. 682	68, 700	100, 100	. 050	. 009	9. 24	11.50	4	. 1035	. 5785	82.0	3. 51	J
A-99b	1.800	. 682	68,000	99, 800	. 050	. 009	9. 23	11.55	4	. 1040	. 5780	82. 0	3. 51	J
A-100a	1.462	. 551	55, 040	100,000	. 050	. 009	9. 26	13.06	3	. 1175	. 4335	72. 8	6.02	0
A-100b	1.464	. 5545	54, 780	99,000	. 050	. 009	9. 24	13. 05	3	. 1174	. 4371	72.8	6.02	g
A-101a	1.004	. 3795	35, 750	94, 200	. 050	. 009	9. 25	10.35	2	. 0932	. 2863 . 2844	63. 0 62. 6	9. 37 9. 37	G
A-101b	1.000	. 378	37, 150	98, 250	. 050	. 009	9. 25 9. 22	10.40	3 2 2 5 5	. 1445	. 7215	81. 6	2. 51	Ğ
A-102a	2. 285	. 866	100, 900	116, 400	. 050	. 013	9. 22	11. 12 11. 05	0 8	. 1445	7213	81. 6	2. 51 2. 51	Ğ
A-102b	2. 285	. 866	97, 500	112, 700	. 050	. 013	9. 22	13.88	4	. 1942	. 5798	70. 6	4. 30	1
A-103a	2. 052	. 774	62, 400 75, 910	80, 600 98, 500	. 050 . 050	.014	9. 26	13. 86	4	. 1940	. 5760	70. 6	4. 30	Ğ
A-103b A-104a	2. 041 1. 671	. 632	45, 800	72, 500	. 050	.014	9. 24	14.44		. 2020	. 430	60. 5	6, 69	ŏ
A-1048	1.658	.627	55, 050	88,000	. 050	. 014	9. 24	14. 40	3	. 2015	4255	60. 2	6. 69	G
A-1040	1. 164	. 440	37, 440	85, 000	. 049	. 014	9. 24	11.04	2	. 1546	. 2854	50. 4	10.03	G
A-105h	1. 167	. 441	35, 600	80, 700	. 050	.014	9. 25	11.06	2 2 6	. 1548	. 2862	50. 6	10.03	J
A-106a	2. 762	1. 051	113, 300	107, 800	. 050	. 018	9. 20	9. 87	6	. 1778	. 8732	82. 2	1.76	J
A-106b	2. 760	1.047	120, 100	114, 800	. 050	. 018	9. 22	9.80	6	. 1765	. 8705	82. 1	1.76	G
A-107a	2, 021	. 765	74,650	97, 600	. 050	. 018	9. 22	10.05	4	. 1810	. 584	73. 0	3. 01	J
A-107b	2. 024	. 766	73,070	95, 500	. 050	. 018	9. 24	10.05	4	. 1810	. 585	72. 9	3. 01	î
A-108a	1.665	, 6305	57, 140	90, 500	. 050	. 018	9. 23	10.42	3	. 1877	. 4428	63. 7	4.60	l î
A-108b	1.658	. 628	55,860	89,000	. 050	. 018	9. 23	10.43	3	. 1879	. 4401	63. 5	4. 69	J
A-109a	1. 167	. 442	25,000	56,600	. 050	. 018	9. 22	8.06	2	. 1452	. 2968	54.0	7.03	1
A-109b	1. 153	. 437	38, 300	87,600	. 050	. 018	9. 22	8.06	2 2	. 1452	. 2918	53. 5 43. 7	7. 03 10. 54	ď
A-110a	1.330	5025	39, 720	89,000	. 050	. 018	9. 25	11. 55	2	. 2080 . 2420	. 2945	39. 9	10. 54	ă
A-110b	1. 421	. 536	43,900	81,900	. 050	. 021	9. 26	11. 54 11. 04	2 6	. 3205	. 8665	71. 2	2, 01	Ğ
A-111a	3. 136	1. 187	129, 300	109,000	. 050	. 029	9. 24 9. 23	11.04	6	. 3195	. 8605	71. 1	2. 01	Ĭ
A-111b	3. 116	1. 180	131, 300	111,300 105,700	. 050	.029	9. 23	10. 37	4	.3010	. 5780	61. 5	3. 12	Ĵ
A-112a	2. 320	.879	92,900	99,000	. 050	. 029	9. 23	10. 38	1	. 3013	. 5797	61. 5	3, 12	Ĵ
A-112b A-113a	2, 326 1, 945	. 881 . 735	87, 300 66, 390	90,000	. 050	. 029	9. 25	10. 36		3005	. 4345	51. 5	4, 68	F
A-113a	1. 943	. 734	68, 480	93, 200	. 050	. 029	9, 22	10.36	3	. 3003	. 4337	51. 5	4.68	G
A-114a	1. 391	. 525	43, 650	83,000	.050	. 029	9. 26	8.06	2	. 2340	. 2910	41.6	7. 03	G
A-114b	1. 392	. 525	47, 180	90,000	. 050	. 029	9. 26	8. 07	3 2 2 2 2 2	. 2340	2910	41.6	7. 03	Q
A-115a	1.699	. 641	44, 820	69, 700	. 050	. 029	9. 25	11.97	2	. 3475	. 2935	31.5	10. 93	G
A-115b	1.592	. 6395	45, 100	70, 800	. 050	. 029	9. 25	11.96	2	. 3470	. 2925	31.5	10. 93 1. 87	G
A-116a	3.648	1.380	181,900	131,800	. 050	. 048	9. 24	10.38	6	. 4985	. 8815 . 8755	62. 0 61. 9	1.87	í
A-116b	3. 630	1.375	173, 200	126,000	. 050	. 048	9. 23 9. 22	10. 40 9. 45	6	. 4995	. 5910	52. 2	2. 81	5
A-117a	2. 757	1.045	116, 400	111, 400	. 050	.048	9, 22	9. 44	1	4530	. 5890	52. 1	2. 81	Ğ
A-117b	2. 751	1.042	117, 500 76, 100	112,800 84,200	. 050	.049	9. 22	9, 50	3	4650	. 4400	41.5	4, 22	1
A-118a A-118b	2. 385 2. 377	.900	95,000	105, 600	. 050	.048	9, 22	9.48	3	. 4550	4450	42.2	4. 22	G
A-1180 A-119a	2, 986	1, 130	65, 300	57, 800	. 050	. 048	9. 24	14. 14	3	. 6790	. 4510	32.3	6.56	J
A-119b	2, 988	1. 130	80, 200	71,000	. 050	. 048	9. 25	14. 16	3	, 6795	. 4505	32. 3	6. 56	J
A-119Aa	2. 365	. 895	52, 500	58, 700	. 050	. 049	9. 24	12. 29	2	. 6015	. 2935	21.0	11. 24	G
A-119Ab	2. 359	. 895	46,000	51, 400	. 050	. 049	9. 22	12.30	2	. 6025	. 2925	21.0	11. 24	J

 $\begin{array}{c} \text{Table IX} \\ \text{[Stiffener pitch=2.50 inches]} \end{array}$

			77 - 111	Failing	Thick	eness			Num-			Percent	Туре
Specifica- tion no.	Weight (pounds)	Area (square inch)	Failing load (pounds)	stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	ber of stiffen- ers	Area of sheet	Area of stiffeners	rein- force- ment	of failure
B-120a B-120b B-121a B-121a B-122b B-123b B-123b B-123b B-123b B-125a B-125a B-125a B-126a B-126a B-127a B-127a B-127a B-128b B-130a B-130a B-130b B-131a B-131a B-131b B-133a B-133b B-133a B-133b B-133b B-133b B-133b B-133b B-133b B-133b B-133b B-133b B-134b	0. 492 .484 .714 .700 .939 .946 1. 380 1. 377 .767 .981 .987 1. 210 1. 264 1. 496 1. 496 1. 496 1. 496 1. 496 1. 498 1. 498 1. 772 1. 745 2. 011 2. 011 2. 011 2. 011 2. 012	0. 1863	10, 870 10, 570 11, 5950 15, 920 20, 6900 21, 400 33, 1900 28, 910 28, 910 28, 900 42, 200 42, 200 42, 200 42, 200 43, 720 44, 1, 100 48, 820 59, 900 77, 230 86, 690 85, 777, 330 86, 690	88, 300 57, 800 59, 000 59, 000 58, 900 60, 000 62, 400 62, 400 94, 900 92, 000 92, 000 93, 300 99, 100 103, 600 103, 600 107, 000 100, 200 114, 000 98, 500 114, 000 91, 100 101, 800 101, 800 101, 800	0. 010	0.009 .009 .018 .018 .029 .029 .048 .009 .019 .029 .029 .048 .048 .009 .018 .029 .029 .048 .048 .048 .048 .048 .048 .048 .048 .048 .048 .048 .048 .049 .048 .049 .048 .048 .049 .048 .049 .049 .049 .049 .049 .049 .048	9. 23 9. 22 9. 24 9. 24 9. 25 9. 26 9. 25 9. 24 9. 23 9. 23 9. 24 9. 23 9. 24 9. 23 9. 24 9. 25 9. 25 9. 25 9. 25 9. 25 9. 25 9. 27 9. 27	8. 40 8. 39 8. 41 8. 39 8. 38 8. 38 88 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	0. 0756 .0755 .1614 .1510 .2436 .2433 .4020 .4015 .1596 .2430 .2440 .4020 .4020 .4020 .7755 .0754 .1508 .1510 .2430 .2427 .4020 .4020 .4020 .4020 .7754 .1588 .1510 .2427 .4020 .402	0.1107 1079 1188 1136 11107 11195 11195 2140 2123 2142 2150 2215 2210 3220 3220 3220 3220 3220 3220 3220	55. 2 54. 5 39. 7 38. 7 27. 6 28. 2 20. 0 20. 0 70. 3 52. 8 42. 6 42. 6 42. 6 42. 1 31. 5 78. 2 78. 2 78. 2 78. 1 64. 2 64. 3 64. 3 65. 1 73. 6 67. 3 66. 3	FFFFFFFFAAABBFFFEEJJJFFFFJJ
B-135a B-135b	2. 638 2. 678	. 9950 1. 0110	107, 400 106, 800	108, 000 105, 700	. 049	. 049 . 050	9. 26 9. 26	8, 55 8, 54	4	. 4190 . 4265	. 5760 . 5845	54. 0 53. 9	G

TABLE X

					Thic	kness								
Specifica- tion no.	Weight (pounds)	Area (square inch)	Failing load (lbs.)	Failing stress (lb./sq. in.)	Stiff- ener (inch)	Shect (inch)	Length (inches)	Width (inches)	Num- ber of stiffen- ers	Area of sheet	Area of stiff- eners	Stiff- ener pitch (inches)	Percent rein- force- ment	Type of failure
$\begin{array}{c} \text{C-}136a\\ \text{C-}136b\\ \text{C-}136b\\ \text{C-}136b\\ \text{C-}136b\\ \text{C-}136b\\ \text{C-}138b\\ \text{C-}138b\\ \text{C-}138b\\ \text{C-}149b\\ \text{C-}141b\\ \text{C-}141b\\ \text{C-}141b\\ \text{C-}141b\\ \text{C-}141b\\ \text{C-}142b\\ \text{C-}142b\\ \text{C-}143a\\ \text{C-}1445b\\ \text{C-}1445b\\ \text{C-}1445b\\ \text{C-}1445b\\ \text{C-}1445b\\ \text{C-}1445b\\ \text{C-}1445b\\ \text{C-}1445b\\ \text{C-}1445b\\ \text{C-}145a\\ \text{C-}145a\\ \text{C-}145a\\ \text{C-}145a\\ \text{C-}145a\\ \text{C-}145a\\ \text{C-}145b\\ \text{C-}145b\\ \text{C-}145b\\ \text{C-}145b\\ \text{C-}145b\\ \text{C-}145b\\ \text{C-}145b\\ \text{C-}145b\\ \text{C-}150b\\ \text{C-}155a\\ \text{C-}155a\\ \text{C-}155b\\ \text{C-}1$	0.178 .175 .440 .442 .668 .671 .896 .131 .128 .230 .227 .318 .495 .648 .189 .648 .189 .648 .181 .313 .311 .464 .451 .461 .367 .639 .809 .809 .818 .1367 .1.812 .1.812 .1.812 .1.813 .1.1812 .1.813 .1.1812 .1.813 .1.1812 .1.813 .1.1812 .1.813 .1.1812 .1.813 .3318 .331	0. 1700 1656 1670 1683 1670 1683 1692 1694 1245 1245 1227 1241 1225 1227 1804 1227 1804 1720 1698 1127 1703 1703 1703 1703 1703 1703 1703 170	7, 010 5, 870 5, 920 (1) 4, 000 (2) 6, 940 6, 940 6, 940 6, 760 5, 430 5, 325 5, 325 5, 325 5, 325 5, 325 6, 421 14, 670 14, 670 14, 670 14, 670 14, 670 11, 420 11, 420 11, 420 11, 420 11, 420 11, 420 11, 420 11, 420 11, 420 11, 420 11, 431 11, 430 11, 440 11, 440 11, 440 11, 450 1	41, 200 35, 390 35, 400 (1) 23, 600 (1) 55, 400 41, 400 44, 400 44, 400 44, 400 42, 100 52, 700 55, 400 51, 200 42, 100 33, 300 43, 600 42, 100 33, 500 34, 600 42, 100 51, 100 33, 500 34, 600 42, 600 42, 600 42, 600 42, 600 42, 600 42, 600 42, 600 42, 600 42, 600 42, 600 42, 600 43, 600 44, 600 48, 600 48, 600 48, 600 48, 600 48, 600 48, 600 48, 600 48, 600 48, 600 48, 600 48, 600 48, 600 48, 600 48, 600 48, 600 48, 600 48, 600 48, 600 51, 100 38, 300 48, 600 51, 100 38, 300 48, 600 51, 100 71, 500	0.010 0.010	0.009 0.009	66705288885550868444222888474868844432288854886444228874446788444888444328853888884442288873446787444	12. 047 12. 071 12. 07	22222222222222222222222222222222222222	0. 1084 1083 1086 1086 1088 1204 1204 1204 1204 1204 1204 1204 1204	0. 0616 0.0573 0.0573 0.0573 0.0573 0.0574 0.0604 0.0604 0.0580 0.0580 0.0582 0.0572 0.0534 0.0581 0.0584 0.0581 0.0582 0.0865 0.0865 0.0865 0.0876 0.0870 0.0870 0.0870 0.0920 0.0913 0.0920 0.0913 0.0940 0.0920 0.0913 0.0940 0.0920 0.0913 0.0940 0.0920 0.0913 0.0940 0.0920 0.0913 0.0940 0.0920 0.0913 0.0940 0.0920 0.0913 0.0940 0.0920 0.0913 0.0940 0.0920 0.0913 0.0940 0.0920 0.0930 0.0940 0.0920 0.0913 0.0940 0.0920 0.0913 0.0940 0.0920 0.0913 0.0940 0.0920 0.0913 0.0940 0.0920 0.0913 0.0940 0.0920 0.0913 0.0940 0.0920 0	11. 16 11	23. 5	A FF G AAFFFFGG AAAAHH GGGAAAAHAJHFFFFFFFFFFFFFGGGGAAAAFGGGHHHAAEEAGGGGFEAEAAG GFFFGEE

[!] Too flimsy.

TABLE X-Continued

Specifica- tion no.	Weight (pounds)	Area (square inch)	Failing load (lbs.)	Failing stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	Num- ber of stiffen- ers	Area of sheet	Area of stiff- eners	Stiff- ener pitch (inches)	Percent rein- force- ment	Type of failure
C-174a C-174a C-174a C-175a C-1756 C-1756 C-176b C-1776 C-1776 C-1776 C-1776 C-1776 C-178a C-178a C-178b C-1816 C-	1. 281 1. 925 1. 926 2. 557 6. 688 1. 122 1. 123 1. 1. 124 1.	1 485:4 61224 61234 61224 61234 61234 61234 61234 61234 61234 61234 6134 6134 6134 6134 6134 6134 6134 61	55, 900 51, 930 51, 940 53, 890 45, 300 61, 690 43, 866 40, 955 41, 740 41, 777 61, 61, 62, 62, 62, 62, 62, 62, 62, 62, 62, 62	6 68, 800 6 81, 902 6 92, 256 6 84, 800 6 87, 300 6 87, 300 6 88, 600 7 72, 700 6 72, 700 6 74, 100 6 72, 700 6 74, 100 6 72, 700 6 74, 100 7 70, 900 7 70, 900 8 70, 900 9 8, 500 9 10, 500 9 11, 500 9 116, 500 9 117, 500 9 118, 500 9 1	0 0 020 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 0220 0 0 0 0220 0 0 0 0	3. 63 6. 46 6. 42 42 6. 44 42 6. 42 23 6. 44 6. 42 23 6. 44 6. 42 23 6. 42 6.	9.28 9.28 9.28 9.31 9.29 9.29 9.28 9.29 9.29 9.29 9.20 9.29 9.20 9.29 9.20 10.22 10.22 10.22 10.22 10.22 10.22 10.22 10.22 10.22 10.22 10.22 10.22 10.23 10.24 10.25 10.23 10.26 10.27 10.26 10.27 10.	333334444444444444444444444444444444444	1671 1763 1673 1673 1671 2048 1047 1849 1941 1841 1941 1941 1941 1941 1941	0 3256 0 3266 0 3246 0 3246 0 3246 0 3255 0 3255 0 3256 0 3240 0 4360 0 4360 0 4300 0 4300	2.80 2.80 2.80 2.80 2.80 1.87 1.87 1.87 1.87 1.87 1.87 1.87 1.87	51. 9 51. 9 50. 0 42. 7 52. 3 50. 0 52. 3 50. 1 52. 3 50. 1 52. 3 61. 5 62. 3 60. 6 60. 6 60	HHHHIJFJGGGDDDFJJJJGJGF FF

TABLE X—Continued

					Thic	kness								
Specifica- tion no.	Weight (pounds)	Area (square inch)	Failing load (lbs.)	Failing stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	Num- ber of stiffen- ers	Area of sheet	Area of stiff- eners	Stiff- ener pitch (inches)	Percent rein- force- ment	Type of failure
C-210h C-211a C-211b C-211b C-211b C-211b C-212b C-213a C-212b C-213a C-214b C-214a C-214b C-215b C-216a C-216a C-216a C-216a C-216a C-216a C-217a C-76b C-218a C-218a C-219a C-221b C-221a C-221b C-221a C-221b C-221a C-221b C-221a C-221b C-222a C-221b C-223a C-223b C-223a C-223b C-233a C-233a C-233b C-233a C-233b C-234b C-234b C-234b C-234b C-234b C-234b C-241a C-241a C-241a C-241a C-241a C-241a C-241a C-241a C-241a C-241b C-241a C-241b C-241a	1. 133 1. 509 1. 450 1. 450 1. 451 1. 458 1. 324 1. 304 1. 377 1. 370 1. 655 1. 966 1. 966 1. 966 1. 966 1. 966 1. 966 1. 967 1. 409 1. 100 1.	5910 5910 5920 5880 5900 5910 4935 5925 770 7725 7725 77690 7785 7650 7690 7690	65, 740 56, 450 60, 300 56, 500 55, 300 41, 080 46, 420 35, 810 92, 400 89, 100 92, 000 91, 700 86, 100	104, 400 107, 000 98, 500 94, 100 82, 300 63, 800 70, 300 44, 000 446, 400 99, 000 84, 500 66, 300 50, 600 66, 300 105, 200 105, 200 106, 200 107, 200 108, 200 109, 200 109, 200 101, 200	0. 030 0.031 0.031 0.031 0.031 0.031 0.030	0.009 0.009 0.009 0.009 0.009 0.009 0.009 0.009 0.009 0.009 0.009 0.009 0.011 0.011 0.019 0.018	13. 85. 18. 5. 1	13. 53 13. 54 19. 32 9. 33 19. 34 19. 31 19. 32 19. 33 19. 34 19. 31 12. 12 12. 12 12. 12 12. 12 12. 12 12. 12 12. 12 12. 12 12. 12 12. 13 13. 54 13. 55 13.	222222222233333333333333333333333333333	0. 1218 1220 1220 0839 0840 0837 0840 0837 0840 0838 0840 0841 0838 1091 1093 1092 1091 1091 1091 1091 1091 1091 1091	0. 1642 1615 1610 1621 1603 1621 1603 1623 1623 1623 1623 1629 1623 2449 2442 2444 2447 2380 2394 2413 2424 2447 2380 2349 2443 2447 2447 2380 245 245 245 245 245 247 247 248 248 249 248 248 248 248 248 248 248 248 248 248	12.65 12.65	41. 5 4 4 5 1. 7 3 5 1. 3 9 6 6 1. 5 5 5 2. 2 7 7 9 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6	АНООГОООГЕГЕГЕГЕСООООГНЕГЕВООООГЕГЕГЕВВООООГНЕГЕВВООООГЕГЕГЕННОООООГНЕГЕННООООООГНЕГЕННОООООООГНЕГЕННОООООООГНЕГЕННОООООООО

Table XI.—Stiffeners without sheet attached

G	337.1.1.4	Area	Failing	Failing	Thickness	1	
Specifica-	Weight	(square	load	stress	of stiffener	Length	71
tion no.	(pounds)	inch)	(pounds)	(lb./sq.in.)	(inch)	(inches)	L/ ho
		ILCII)	(pounds)	(1D./sq. 1L.)	(inch)	(,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
D-244a	0, 030	0.0000	0.000	00.700		-	
D-244a	. 029	0. 0283	2,650	93, 700	0.011	3. 70	18. 5
D-245a	.029	. 0274	2, 420	88, 400	. 011	3. 70	18. 5
D-245a D-245b		. 0276	1,660	60, 100	. 011	6. 46	32. 2
	. 051	. 0276	1,980	71,700	.011	6. 46	32. 2
D-246a	. 073	. 0276	1,200	43,500	.011	9. 24	46. 0
D-246b	. 073	. 0276	1,420	51, 400	.010	9. 25	46.1
D-247a	. 110	. 0288	835	29,000	.010	13. 87	69. 2
D-247b D-248a	. 110	. 0288	900	31, 200	.010	13. 87	69, 2
D-2488 D-248b	. 155	. 0292	550	18,800	.010	18. 50	92, 2
	. 151	. 0285	550	19, 300	.010	18. 50	92. 2
D-249a	. 043	. 0406	3,800	93, 500	. 015	3.70	18. 5
D-249b	. 043	. 0406	4,300	105, 800	. 015	3.70	18. 5
D-250a	. 074	, 0400	2,920	7, 300	. 015	6, 46	32, 2
D-250b	. 074	. 0400	3, 020	75, 500	. 015	6. 47	32, 2
D-251a	. 110	. 0415	2, 310	55, 600	. 015	9. 25	46. 1
D-251b	. 107	. 0405	2, 140	52, 900	. 015	9. 24	46. 0
D-252a	. 158	. 0398	1, 420	35, 600	. 015	13. 88	69. 2
D-252b	. 166	. 0434	1, 440	34, 600	. 015	13, 88	69. 2
D-253a	. 220	. 0416	1,030	24, 800	. 015	18, 50	92, 2
D-253b	. 222	. 0419	1, 100	26, 200	. 015	18. 50	92. 2
D-254a	. 055	. 052	5, 420	104, 100	. 020	3. 70	18. 5
D-255a	. 096	. 0518	4, 330	83, 600	. 020	6.48	32. 3
D-255b	. 096	. 0519	4, 400	85,000	. 019	6. 49	32. 3
D-256a	. 141	. 0534	2,990	56,000	. 020	9. 24	46. 0
D-256b	. 141	. 0534	3, 240	60, 700	. 020	9. 25	46.1
D-257a	. 214	. 0540	2, 200	40, 700	. 020	13. 86	69. 2
D-257b	. 217	. 0546	2, 540	46, 500	. 020	13. 89	69. 2
D-258a	. 283	. 0555	1,730	31, 200	. 020	18. 48	92. 1
D-258b	. 281	. 0553	1, 895	34, 300	. 020	18. 46	92. 0
D-621a	. 081	.077	10, 470	136, 000	030 1	3.68	18. 4
D-621b	. 081	. 0765	10, 140	132, 800	.030	3. 70	18. 5
D-622a	. 146	. 077	6, 900	90, 100	.030	6.46	32. 7
D-622b	. 146	. 077	7, 300	94, 700	. 030	6, 46	32. 7
D-623a	. 213	. 0805	5,900	73, 400	. 030	9. 25	46. 1
D-623b	. 212	. 0805	5, 600	69, 600	. 030	9. 24	46. 0
D-624a	. 318	. 0800	3, 970	49, 700	030	13. 87	69. 2
D-624b	. 318	. 0800	4,000	50,000	. 030	13. 87	69. 2
D-625a	. 426	. 0805	3, 110	38, 600	. 030	18. 49	92. 1
D-625b	. 427	. 0805	3, 110	38, 600	. 030	18. 51	92. 4
D-626b	. 150	. 1425	19, 800	139, 000	. 049	3, 68	18.4
D-627a	. 265	. 143	15, 100	105, 800	. 048	6, 48	32. 3
D-627b	. 263	. 1415	15, 600	110,000	. 048	6.48	32.3
D-628a	. 380	. 1435	13, 600	94, 600	. 049	9. 26	46. 1
D-628b	. 380	. 1440	13, 100	91,000	. 049	9. 22	45.9
D-629a	. 572	. 1444	9, 850	68, 300	. 049	13. 85	69. 0
D-629b	. 571	. 1440	9, 150	63, 500	. 049	13.88	69. 2
D-630a	. 764	. 1440	7,880	54, 800	. 049	18. 50	92. 2
D-630b	. 760	. 1435	8, 200	57, 100	, 048	18. 49	92.1
					!		

Table XII [Stiffener width, W=0.500 inch. Stiffener pitch=2.50 inches]

					Thic	kness			1			Sug	}	
Specifica- tion no.	Weight (pounds)	Area (square inch)	Failing load (pounds)	Failing stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	Number of stiff- eners	Area of sheet	Area of stiff- eners	Stiff- ener depth (D inches)	Percent rein- force- ment	Type of failure
E A-259a E A-260a E A-261b E A-261b E A-261b E A-262a E A-263b E A-263b E A-263b E A-264b E A-265b E A-265b E A-265b E A-266b E A-268a E A-268a E A-270b E A-270b E A-271b E A-271b E A-272b E A-272b E A-273a E A-273a E A-274b	0. 262 511 515 763 762 415 827 822 1. 236 1. 232 1. 232 1. 181 1. 105 1. 179 1. 179 1. 179 2. 374 1. 067 1.	0. 1980 1927 1937 1911 1910 2108 2108 2108 22085 2080 2298 2255 2222 2440 2623 2623 2650 2420 2780 2780 2780 2780 2780 2780 2783 2783 2783 2783 2783 2783 2783 2783	13, 050 10, 900 12, 000 9, 300 10, 450 11, 700 10, 150 10, 150 10, 150 10, 570 10, 410 10, 350 10, 500 11, 500 10, 600 10, 600	65, 900 56, 600 62, 000 48, 600 54, 700 55, 450 57, 350 52, 600 48, 650 48, 650 48, 600 46, 100 46, 100 46, 250 47, 100 47, 100 42, 250 43, 800 42, 250 38, 400 36, 550 36, 550 36, 550 35, 900 58, 900	0. 010 010 010 010 010 010 010 009 010 009 010 009 009	0. 009 . 009 . 009 . 009 . 009 . 010 . 009 . 000 . 000	4. 63 9. 27 9. 30 13. 94 6. 88 13. 76 13. 79 20. 68 9. 08 9. 15 18. 33 26. 79 11. 28 12. 60 22. 66 33. 93 33. 88 13. 40 26. 80 40. 19 4. 6. 80 40. 19 4. 6. 80 40. 19 4. 6. 80	8, 42 8, 33 8, 40 8, 49 8, 49 8, 45 8, 42 8, 34 8, 42 8, 39 8, 38 8, 42 8, 39 8, 38 8, 42 8, 38 8, 42 8, 38 8, 42 8, 38 8, 42 8, 38 8, 40 8, 80 8, 80	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	0. 0758 .0759 .0756 .0756 .0756 .0768 .0764 .0842 .0779 .0842 .0758 .0838 .0842 .0758 .0755 .0755 .0754 .0751 .0838 .0839 .0840 .0762 .0838 .0839 .0840 .0762 .0838 .0839 .083	0. 1222 .1177 .1181 .1155 .1154 .1344 .1255 .1243 .1335 .1246 .1460 .1416 .1497 .1463 .1473 .1473 .1686 .1698 .1693 .1693 .1822 .1821 .1803 .1822 .1821 .1803 .1822 .1823 .1823 .1824 .1823 .1824 .1823 .1824 .1823 .1824 .1823 .1824 .1823 .1824 .1823 .1824 .1823 .1824 .1823 .1824 .1823 .1824 .1823 .1824 .1823 .1824 .1823 .1824 .1823 .1824 .1824 .1824 .1825 .1822 .1821 .1823 .1824 .1824 .1824 .1825 .1822 .1824 .1825 .1822 .1824 .1825 .1822 .1824 .1825 .1826 .1826 .1826 .1826 .1826 .1826 .1827 .1827 .1828 .1828 .1828 .1829 .1829 .1829 .1821 .1820 .1821 .1822 .1824 .1824 .1825 .1826 .1826 .1826 .1826 .1827 .1827 .1827 .1828 .1829 .1829 .1829 .1829 .1820	6. 500 500 500 500 500 750 750 750	57. 5 56. 7 56. 2 59. 8 55. 7 55. 5 59. 7 55. 5 59. 7 55. 5 62. 9 62. 0 64. 6	FFFGGHAFFFFFFFGGAAAFGGFFGGGGEE

TABLE XII-Continued

[Stiffener width, W=0.500 inch. Stiffener pitch=2.50 inches]

				(Stiffener Wi	Thiel						,	Stiff-		
Specifica- tion no.	Weight (pounds)	Area (square inch)	Failing load (pounds)	Failing stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	Number of stiff- eners	Area of sheet	Area of stiff- eners	ener depth (D inches)	Percent rein- forced	Type of failure
EA-275a EA-275a EA-277a EA-277a EA-277a EA-277a EA-277a EA-278a EA-278a EA-278a EA-278a EA-278a EA-278a EA-280b EA-280b EA-281b EA-281b EA-282a EA-282a EA-282a EA-282b EA-283a EA-282b EA-283a EA-282b EA-283a EA-284b EA-285b EA-286b EA-286b EA-287b EA-287b EA-287b EA-287b EA-287b EA-287b EA-297a EA-297a EA-299a EA-299a EA-299a EA-299a EA-299a EA-299b EA-299b EA-299b EA-299b EA-299b EA-299b EA-299b EA-299b EA-300a EA-300b EA-300b EA-301b EA-301b EA-302a EA-304b EA-307b EA-308b EA-307b EA-308b EA-307b EA-311a EA-311b EA-312b EA-311b EA-312b EA-311b EA-312b	0. 937	0. 3523 3510 3470 3470 3471 3710 3710 3710 3710 38710 38710 3883 3883 3746 3810 3815 3815 3815 3815 3815 3815 3815 3815	58, 450 59, 250 52, 340 48, 070 41, 690 60, 759 58, 720 42, 630 45, 580 60, 590 56, 430 56, 430 56, 430 56, 430 56, 430 56, 430 56, 430 60, 750 60, 750	125, 300 126, 800 126, 800 115, 500 88, 500 115, 300 115, 300 114, 000 88, 250 82, 900 85, 100 104, 300 98, 100 97, 450 77, 500 77, 500 77, 500 77, 500 61, 200 110, 600 110, 600 110, 700		0. 028 028 028 028 028 029 029 029 029 029 029 028 028 028 028 028 028 028 028 028 028	13. 94 6. 87 13. 80 20. 70 9. 15 9. 15 18. 34 18. 33 26. 79 26. 79 21. 26 22. 58 22. 53 33. 82 22. 53 33. 82 24. 33 34. 44. 464 4. 644	8. 44 8. 49 8. 42 8. 42 8. 40 8. 38 8. 38 8. 38 8. 43 8. 43 8. 44 8. 49 8. 40 8. 40 80 80 80 80 80 80 80 80 80 80 80 80 80	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	0844 0848 0849 0757 0842 0757 0840 0758 0754 0754 0758 0759 0759 0759 0759 0759 0759 0759 0759 0759 0759 0757 0764 0755 0764 0752 0752 0752 0754 0754 0755 0754 0755 0758 0759	3826 3825 3811 3818 3778 3893 4425 4402 4506 4466 4482 5052 5071 5031 5055 5041 5794 5794 5794 5793 4796 5833 3167	. 500 . 750 . 750 . 750 . 750 . 750 . 750 . 1. 000 . 1. 000 . 1. 000 . 1. 250 . 1. 500 . 500 . 500 . 500	29. 6 3 3 2 8 8 1 3 2 8 8 1 3 2 8 8 1 3 2 8 8 1 3 2 8 8 1 3 2 8 8 1 3 2 8 8 1 3 2 8 8 1 3 2 8 1 3 3 8 1 3 8 1 3 8 1 3 8 1 3 8 1 3 8 1 8 1	FGGGJGLLLGGGBB

TABLE XII—Continued

[Stiffener width, W=0.500 inch. Stiffener pitch=2.50 inches]

	1	1	1	Γ	ı — —		1	ı	1 - 2.30 11			 	1	ī
Specifica-	Weight	Area	Failing	Failing	Thic	kness	Length	Width	Number	A	Area of	Stiff- ener	Percent	Туре
tion no.	(pounds)	(square inch)	load (pounds)	stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	(inches)	(inches)	of stiff- eners	Area of sheet	stiff- eners	depth (D inches)	force- ment	of failure
EA-320b EA-321b EA-322b EA-332b EA-332b EA-332b EA-333b EA-333b EA-333b EA-333b EA-333b EA-333b EA-334b EA-334b EA-334b EA-342b EA-343b EA-345b EA-356a EA-356a EA-356a EA-366a EA-366a EA-366a EA-366a	1. 475 2. 200 2. 193 1. 2215 2. 514 2. 2515 2. 514 2. 3. 695 3. 765 1. 778 3. 562 2. 405 2. 2405 2. 406 2. 958 3. 762 2. 406 2. 958 3. 762 2. 406 3. 101 3. 100 4. 6675 2. 231 4. 468 4. 481 6. 645 4. 451 6. 544 4. 561 6. 544 6. 545 6. 545 6. 545 6. 545 6. 545 6. 545 6. 546 6. 547 6. 218 6. 218 6.	0. 5545	55, 540 44, 910 35, 000 68, 700 67, 000 73, 850 70, 850 70, 950 45, 000 60, 510 71, 550 71, 950 66, 910 70, 660 60, 910 70, 660 80, 910 70, 660 81, 900 88, 900 88, 1340 62, 175 66, 000 78, 825 87, 700 88, 000 78, 825 87, 700 88, 000 78, 825 87, 150 88, 1340 62, 175 66, 000 77, 470 88, 1340 62, 175 66, 000 77, 150 88	100, 300 81, 500 81, 500 110, 600 102, 300 102, 300 107, 800 107, 800 107, 800 107, 800 107, 800 107, 800 108, 500 88, 500 98, 750 98, 700 98, 400 88, 500 88, 500 98, 700 108, 600 108, 600 108, 600 108, 600 108, 600 108, 600 108, 600 108, 600 108, 600 108, 600 108, 600 108, 600 108, 600 108, 600 108, 600 108, 600 108, 700 108	0.030 0.030	0. 028	9. 31 13. 94 6. 86 6. 87 6. 88 6. 13. 78 13. 79 20. 70 20. 70 20. 67 9. 90 18. 31 11. 30 122. 60 222. 57 33. 89 11. 30 122. 60 223. 79 40. 25 4. 66 4. 66 5. 79 70 71 71 71 71 72 72 73 74 74 76 76 76 76 76 76 76 76 76 76 76 76 76	8.4 44 44 44 44 44 44 44 44 44 44 44 44 4		0. 2346	0. 3199	0.500 -500 -500 -750 -750 -750 -750 -750 -	53. 3 51. 5 56. 5 66. 4 39. 4 43. 5 56. 6 66. 4 47. 5 56. 6 57. 6 58. 5 58. 5 58	0011 0111111111111111111111111111111111

TABLE XII-Continued

[Stiffener width, W=0.500 inch. Stiffener pitch=2.50 inches]

			7	77.11	Thick	ness		Wideh	Number		Area of	Stiff- ener	Percent	Туре
Specifica- tion no.	Weight (pounds)	Area (square inch)	Failing load (pounds)	Failing stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width Width (inches)	Number of stiff- eners	Area of sheet	stiff- ener	depth (D inches)	rein- force- ment	of failure
EA-365a	2. 228 2. 240 3. 335	0.8330	85, 550	102, 900	0.050	0.030	9.35	8. 52	4	0. 2557	0. 5773	0.500	65.8	188888888888888888888888888888888888888
EA-365b	2. 240	. 8370	85, 700	102, 300	. 050 . 050	. 030	9.36 13.93	8. 52	4	. 2557 . 2557 . 2557	. 5813	. 500 . 500	65. 9 65. 9	ă
EA-300a EA-366b	3, 353	. 8390	69, 500	82, 900	. 050	. 030	13. 97	8. 52	4	. 2557	. 5813 . 5833	. 500	66.0	G
EA-367a	3. 353 1. 805	9200	119, 200	129, 600	. 050	. 029	6. 85	8. 52 8. 52 8. 52 8. 51 8. 50	4	. 2465 . 2463 . 2475	6735	. 750	69. 9	G
EA-367b	1 812	.9145	119, 300	130, 600	. 050	. 029	6. 92	8. 50 8. 54	. 4	. 2463 9475	. 6682 . 6735	750	69. 6 69. 8	ă
EA-3688	3. 633 3, 627 5. 463	. 9210	103, 300	112, 400	050	. 029	13. 93 13. 97 6. 85 6. 92 13. 77 13. 79 20. 70	8, 50 8, 58	4	. 2463 . 2487 . 2460	. 6737	. 750	69.8	Ŏ.
EA-369a	5. 463	9225	81,900	88, 900	. 050	. 029 . 029 . 029	20. 70	8. 58	4	. 2487	. 6738 . 6790	. 750	69. 9 70. 0 72. 8	G
EA-369b	5, 480	. 9250	81, 200	87, 900	. 050	. 029	20.68	8. 48 8. 53	4		. 7708	1.000	70.0	j
EA-370a EA-370b	5. 480 2. 673 2. 649	1.0110	133, 700	132, 300	. 050	. 029	9. 17	8. 50	4	. 2463	. 7647	1.000	72. 5	Ğ.
EA-371a	5, 285 5, 338 7, 830	. 8370 . 8370 . 8390 . 9200 . 9145 . 9210 . 9225 . 9250 1. 0180 1. 0110 1. 0080 1. 0180	111, 400	110, 600	. 048	. 028	18. 33	8. 49	4	. 2463 . 2377 . 2377	. 7708 . 7647 . 7703 . 7803	1.000	73.3	G G
EA-371b	5, 338	1.0180	110, 300	108, 300	. 049	. 028	18, 33	8.49	4	2475	7735	1,000	72. 7	Ğ
EA-3728 EA-372b	1 7 738	1.0090	105, 200	104, 200	.048	. 029	26. 80	8. 53 8. 50 8. 49 8. 49 8. 54 8. 54	4	. 2475 . 2390 . 2678	. 7700 . 8792	1.000	72. 5 73. 3 73. 6 72. 7 73. 3 74. 0	G
EA-373a	3. 702 3. 680 7. 419	1. 1470	147, 400	128, 600	. 050	. 031	11. 28	8. 64 8. 54 8. 71	4 4	. 2678	. 8792	1. 250	74. 0 74. 7	1
EA-373b	3.680	1,1380	120 100	125, 800	. 050	. 030	22, 54	8.71	4	. 2560 . 2700	. 8800	1, 250	74.0	Ĵ
EA-374b	7. 416	1. 1480	134, 000	116, 600	.050	.031	22. 59	8. 56	4	. 2655	. 8800 . 8825 . 8835	1. 250	74. 0 74. 0 74. 0	J
EA-375a	7. 416 11. 120 11. 153	1. 1470	107, 500	93, 700	. 050 . 049	. 031	33.87	8.50	4 4	. 2635	8835	1.250		ď
EA-375b EA-3769	4. 683	1, 1500	144, 900	118, 700	. 050	. 031	13, 40	8. 56 8. 50 8. 60 8. 54	4	. 2665 . 2475	. 9735	1. 500	77. 0	Ĵ
EA-376b	4. 683 9. 372	1. 2210	143, 300	117, 300	. 050	. 029	13. 39	8. 52	4	. 2470	. 9740	1.500	77. 0	I
EA-377a	9. 372 9. 559	1, 2220	125, 200	102, 300 85, 300 82, 900 129, 600 130, 600 113, 500 112, 400 88, 900 87, 900 102, 200 132, 300 104, 200 128, 600 128, 600 129, 700 118, 700 117, 300 118, 700 117, 300 118, 700 118, 700 118, 700 118, 700 118, 700 119, 600 125, 600 125, 600 125, 600 125, 600 125, 600 125, 600 125, 600 125, 600 127, 200 111, 800 127, 200 111, 800 127, 200 117, 300 100, 300 101, 300 102, 400 103, 800 105, 600 105, 300 107, 300 108, 300 109, 300 100, 300 100, 300 100, 300 100, 300	. 049	. 030	9. 18 9. 17 18. 33 18. 33 26. 79 26. 80 11. 28 11. 29 22. 59 33. 87 33. 88 13. 39 26. 80 40. 15 40. 17 4. 64	8. 54 8. 52 8. 50 8. 47 8. 54 8. 59 8. 54 8. 57 8. 58 8. 52 8. 52	4	. 2470 . 2550 . 2708 . 2475	.8835 .9735 .9740 .9670 .9752 .9750	750 750 750 750 750 750 750 1.000 1.000 1.000 1.250 1.250 1.250 1.250 1.500 1.500 1.500 1.500	77. 0 77. 0 76. 5 75. 3 74. 7 75. 8 53. 8	G
EA-378a	14. 045	1. 2225	108, 100	88, 400	. 050	. 032 . 029 . 031	40. 15	8. 54	4	. 2475	. 9750	1.500	74.7	ĵ
EA-378b	14. 045 14. 240 1. 312	1, 2380	114, 900	92, 800	. 049	. 031	40. 17	8. 59	4	. 2660	5705	1,500	75. 8 53. 8	4
EA-379a EA-379b	1.312	9890	118, 300	124, 800	. 050	. 049	4, 66	8. 57	4	. 4185 . 4200	. 5705 . 5705	. 500	53. 8 54. 0	Ĭ
EA-380a	2. 653	, 9950	101, 700	102, 400	. 050	. 049	9.30	8. 58	4	1 4203	. 5747	. 500	54. 0 54. 0	¦ G
EA-380b	2. 649	. 9910	103,000	103, 800	. 050	. 049	9.34	8, 52	4	4175	. 5735 . 5685	500	53.6	Ġ
EA-381a EA-381b	1. 312 1. 320 2. 653 2. 649 3. 937 3. 953 2. 168 2. 140 4. 384	. 9940	84, 100	84, 600	. 050	.049	9. 34 13. 26 13. 90 6. 89 6. 88 13. 78 13. 77 20. 68 20. 69	Q 54	4	. 4175 . 4175 . 4185	. 5685 . 5755 . 6760	. 500	54. 0 57. 5	Q.
EA-382a	2. 168	1, 1000	138, 200	125, 600	. 050	. 049 . 050 . 049	6.89	8. 48 8. 51 8. 52	4	4940	. 6760	750	57. 5 57. 8	J
EA-382b	2.140	1.0880	136, 250	125, 200	, 050	049	13. 78	8. 52	4	. 4165 . 4175 . 4195	6745	.750	57. 8 57. 7 57. 7 57. 8 61. 4 61. 0	Ğ
EA-383b	4. 284	1. 0880	118, 650	109,000	. 050	. 049 . 049 . 049	13. 77	9 56	4	. 4195	. 6745 . 6685 . 6705	. 750	57.7	J
EA-384a	4. 284 6. 451 6. 462	1.0900	87, 200	80,000	. 050	.049	20. 68	8.56	4	4195	6730	750	57. 8	j
E A -3840 E A -385a	3. 134	1, 1970	137, 800	115, 200	. 050	. 049	9, 16	8. 56 8. 56 8. 54	4	. 4185	. 6730 . 7785 . 7660	1.000	61.4	J
EA-385b	3. 081	1. 1800	162, 500	137, 600	. 050	. 049	9, 13	8, 45	4	. 4140	. 7660	1.000	61.0	H
EA-386a	3. 081 6. 294 6. 280 9. 114 9. 226 4. 185 4. 195 8. 427 8. 414 12. 744 12. 644 5. 381	1. 0210 1. 0090 1. 1470 1. 1380 1. 1470 1. 1380 1. 1470 1. 1500 1. 2210 1. 2220 1. 2400 1. 2225 1. 2380 9905 9910 9880 1. 9940 1. 1000 1. 0880 1. 0920 1. 1880 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 1980 1. 1990 1. 2960 1. 3040 1. 3050 1. 3050 1. 4020 1. 4020 1. 4020 1. 4020 1. 4020 1. 4030	85, 550 85, 700 71, 400 71, 400 71, 400 119, 200 119, 300 110, 300 110, 300 81, 200 124, 300 133, 700 111, 400 110, 300 101, 300 111, 400 110, 300 124, 200 120, 100 134, 000 125, 200 134, 900 144, 900 144, 300 181, 300 181, 300 182, 200 183, 300 184, 300 183, 300 184, 300 185, 200 181, 300 181, 300 183, 300 184, 300 185, 200 181, 300 181, 300 183, 300 184, 300 185, 200 186, 500 187, 800 188, 300 188, 300 188, 300 188, 300 188, 300 188, 300 188, 300 188, 300 188, 300 188, 300 188, 300 188, 300 188, 300 189, 300 189, 300 181, 650 182, 300 184, 300	111, 800	. 050	. 049	9. 16 9. 13 18. 34 18. 33 26. 72 26. 79 11. 28 11. 29 22. 57 22. 58 33. 91 13. 42	8. 45 8. 53 8. 53 8. 64	4	. 4195 . 4195 . 4185 . 4140 . 4180 . 4230	. 7810 . 7800	1.000	61. 4 61. 1	Ĵ
EA-387a	9. 114	1. 1930	105, 100	111, 800 88, 000 90, 900 112, 900 118, 800 97, 300 109, 000 93, 200	. 049 . 050 . 050	0.49	26. 72	8. 64	4	. 4230	. 7700 . 7880	1.000	61. 1	l G
EA-387b	9. 226	1. 2030	109, 300	90,900	. 050	.051	26. 79	9 49	4		1 2735	1, 000	60. 6 64. 0	j
EA-3888 EA-388b	4, 185	1. 2960	154, 300	118, 800	0.50	. 049	11. 29	8. 63 8. 62 8. 70	4	4220	. 8760	1. 250	64. 1	ĵ
EA-389a	8. 427	1. 3040	126, 900	97, 300	. 050 . 050 . 050	. 049	22. 57	8. 70 8. 64	4	. 4225 . 4220 . 4260 . 4230	. 8760 . 8780 . 8800 . 8785	1. 250	64. 1 64. 2 64. 2 64. 2	J
EA-389b	8. 414	1.3030	142, 100	93, 200	. 050	. 049 . 049 . 049	33, 91	8, 93	4	. 4375	. 8785	1. 250	64. 2	Ğ
EA-390b	12. 644	1. 3050	106, 700	81, 7	1 050	. 049	33. 91	8.64	4	. 4375 . 4230 . 4290	9820	1. 250	64. 2	J
EA-391a	5. 381	1. 4020	149, 50	106, 800	. 050 . 050 . 050	0.50	13. 42	. 8.58 8.53	4	4190	. 9730 . 9820 . 9785	1,500	66. 0 66. 7 66. 7	Ĭ
EA-391b	5. 355 10. 700	1. 4000	150, 300	107, 600	. 050	. 049 . 049 . 050	26. 70	8. 54 8. 48	4	. 4185	. 9785	1.500	66. 7	J
EA-392b	1 10, 709	1.4030	133, 300	95, 000	. 049	. 050	26. 70	8. 48	4	. 4240	. 9790 . 9825	1.500	63.3	1 1
E A - 345a E A - 345a E A - 345a E A - 365b E A - 366a E A - 366a E A - 366a E A - 367a E A - 367a E A - 368a E A - 368a E A - 370a E A - 370a E A - 371a E A - 372a E A - 372a E A - 372a E A - 372a E A - 373a E A - 373a E A - 375a E A - 385a E A - 386a E A - 387a E A - 387a E A - 389a E A - 390a	16. 115 16. 070	1. 4025 1. 3980	142, 100 122, 500 106, 700 149, 50 164, 800 150, 300 133, 300 101, 200 129, 800	93, 200 81, 7 106, 800 117, 700 107, 600 95, 000 72, 200 92, 800	. 049	. 049	13. 42 13. 37 26. 70 26. 70 40. 17 40. 17	8. 57 8. 64	4	. 4185 . 4240 . 4200 . 4230	. 9750	. 500 . 500 . 500 . 750 . 750 . 750 . 750 . 750 . 750 . 1. 000 1. 000 1. 000 1. 250 1. 250 1. 250 1. 250 1. 250 1. 500 1. 500	61.3 66.8 66.7	j
EA-393D	10.070	1. 3580	120,000	52, 500	.075		10.11	0.01	1	1			1	<u> </u>

TABLE XIII [Stiffener pitch=2.50 inches. Stiffener depth, D=0.500 inch]

7	***	Area	Failing	Failing	Thic	kness			Num-		Arcast	Stiffener	Percent	_
Specifica- tion no.	Weight (pounds)	(square inch)	load	stress (lb./sq.in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	ber of stiff- eners	Area of sheet	Area of stiff- eners	width (W- inches)	rein- force- ment	Type of failure
EB-394a EB-394a EB-394a EB-394a EB-394a EB-394b EB-395b EB-395b EB-396b EB-396a EB-400a EB-400a EB-400a EB-400a EB-405a EB-410b EB-411b EB-411a EB-411b EB-411a EB-411b EB-41b EB-411b EB-411b EB-411b EB-411b EB-411b EB-411b EB-411b EB-411b	0. 264	0. 1998 . 1997 . 2016 . 1998 . 1997 . 2020 . 2015 . 2005 . 2015 . 2005 . 2016 . 2016 . 2140 . 2126 . 2126 . 2140 . 2240 . 2260 . 2440 . 2265 . 2393 . 2405 . 2393 . 2405 . 3700 . 3740 . 3790 . 3740 . 3790 . 3740 . 3000 . 4050 . 4050 . 4075 . 4060 . 4075 . 4060 . 4075 . 4060 . 4075 . 4060 . 4075 . 4060 . 5060 .	12, 300 12, 500 10, 480 11, 380 11, 380 11, 380 11, 800 12, 100 10, 630 10, 630 10, 630 10, 630 10, 630 10, 630 10, 630 10, 630 10, 630 10, 630 11, 200 11, 200 11, 200 11, 200 11, 200 11, 200 11, 200 11, 200 11, 200 12, 600 12, 600 12, 600 12, 600 12, 600 12, 600 12, 600 12, 600 12, 600 12, 600 12, 600 12, 600 12, 600 12, 600 13, 600 14, 850 12, 700 14, 850 15, 550 16, 600 17, 250 18, 600 18, 600 19, 600 19, 600 19, 600 19, 600 19, 600 19, 600 19, 600 19, 600 19, 600 19, 600 19, 600 11, 700 11, 700 11, 700 12, 700 12, 700 12, 700 13, 70	61, 550 62, 560 62, 450 63, 454, 400 56, 100 55, 680 56, 800 56, 800 56, 800 56, 800 54, 250 58, 450 58, 450 58, 460 61, 100 61, 100 61, 100 61, 100 61, 100 61, 100 61, 250 68, 800 68, 800 68, 800 69, 800 60, 800 60, 800 60, 800 60, 800 61, 800 62, 200 63, 800 64, 800 64, 800 64, 800 64, 800 65, 200 66, 800 66, 800 66, 800 66, 800 66, 800 66, 800 66, 800 66, 800 66, 800 66, 800 66, 800 66, 800 66, 800 66, 800 66, 800 67, 500 68, 800 69, 800 60, 800 6	0. 009 0.010 0.010 0.010 0.010 0.009 0.009 0.010 0.010 0.009 0.010	0.009 0.009 0.010 0.010 0.010 0.010 0.010 0.010 0.010 0.010 0.009	4. 62 9. 25 13. 88 13. 85 4. 69 9. 18 13. 34 4. 48 8. 58 12. 99 13. 78 14. 42 4. 62 14. 62 15. 62 16. 62 17. 63 18. 86 19. 19. 19. 19. 19. 19. 19. 19. 19. 19.	8. 646 8. 8. 849 9. 9. 12 12 9. 9. 43 9. 9. 9. 43 8. 8. 8. 8. 8. 8. 8. 8. 8. 8. 8. 8. 8. 8		0. 0776 0. 0871 0. 0864 0. 0878 0. 0864 0. 0879 0. 0868 0. 0878 0. 0879 0. 0889 0. 0889 0. 0889 0. 089	0. 1222 .1127 .1131 .1240 .1151 .1325 .1328 .1243 .1243 .1243 .1244 .1254 .1559 .1560 .1560 .1560 .1560 .1600 .1108 .1148 .1151 .1331 .1261 .1381 .1262 .1280 .1383 .1382 .1290 .1318 .1168 .1148 .1151 .1353 .1202 .1233 .1360 .1414 .1419 .1414 .1459 .1416 .1418 .1150 .117	0.75 .76 .775 .776 .776 .776 .776 .776 .7	57. 6 53. 1 5 53.	БЕААОООНААООБЕАООНЕООООВЕННОСЕЕСНЕ ЕЕСОООЕЕССЕЕННААННОСОДОООСОООООООО ООООООНОООООООООООООО

TABLE XIII—Continued

[Stiffener pitch=2.50 inches. Stiffener depth, D=0.500 inch]

Specifica-	Weight	Area (square	Failing load	Failing	Thic	kness	Length	Width	Num- ber of	Area of	Area of	Stiffener width	Percent rein-	Туре
tion no.	(pounds)	inch)		stress (lb./sq.in.)	Stiff- ener (inch)	Sheet (inch)	(inches)	(inches)	stiff- eners	sheet	stiff- eners	(W- inches)	force- ment	of failure
EB-440b EB-441a EBB-441a	1. 298 1. 298 1. 298 1. 298 1. 933 1. 933 1. 933 1. 803 1. 803 1. 803 1. 802 1. 625 2. 414 2. 421 2. 421 2. 421 2. 585 1. 674 2. 682 2. 545 2. 555 2. 617 2. 905 2. 617 2. 905 2. 617 2. 905 2. 617 2. 905 2. 617 2. 905 2. 617 2. 905 2. 617 2. 905 2. 617 2. 103 2. 2057 2. 103 2. 103 2. 103 2. 105 2. 103 2. 105 2	. 8650 . 8690 . 8665 . 8700 . 8775 . 8800 . 8840 . 8835 . 8810 . 9350 . 9355 . 9405 . 9380 . 9605 . 9500 . 1,0090	44, 086 45, 430 38, 090 40, 890 40, 890 66, 120 66, 310 59, 830 41, 930 66, 620 64, 610 64, 030 65, 100 65, 310 65, 620 66, 530 66, 621 66, 030 66, 100 67, 1650 68, 460 67, 130 68, 460 67, 130 68, 460 67, 130 68, 460 67, 130 68, 460 67, 130 68, 460 67, 130 68, 460 67, 130 68, 460 67, 130 68, 460 67, 130 68, 460 67, 130 68, 460 67, 130 68, 460 69, 100 60, 1	83, 400 85, 700 87, 700 87, 700 87, 700 87, 700 87, 8000 97, 900 97, 900 97, 900 97, 900 97, 900 98, 35, 250 101, 300 100, 700 97, 400 88, 900 91, 300 88, 500 91, 300 91, 300 88, 500 91, 300 101, 500 101, 500 105, 200 116, 300 107, 600 88, 500 108, 800 108, 800 109, 700 88, 900 107, 600 108, 900 109, 200 100, 200 101, 200 101, 200 102, 600 103, 900 104, 200 108, 700 98, 700 88, 500 109, 200 100, 300 100, 300 101, 500 101, 500 102, 600 103, 900 104, 200 108, 500 109, 200 100, 300 100, 300 101, 300 101, 300 101, 300 102, 500 103, 900 61, 750 68, 500 103, 900 61, 750 68, 400 122, 700 88, 500 107, 400 188, 500 107, 400 188, 500 109, 200 101, 300 101, 300 101, 300 101, 500 102, 600 103, 900 104, 200 108, 500 109, 200 109, 200 101, 300 101, 300 101, 500 102, 500 103, 900 104, 300 107, 400 108, 500 109, 200 101, 500 101, 500 102, 500 103, 900 104, 500 105, 600 105, 600 107, 400 108, 500 108, 500 109, 200 111, 600 111, 600 111, 600	0.030 0.030	0.039 0.010 0.039 0.029 0.030	8. 56 8. 56 8. 56 8. 56 9. 31 13. 85 9. 20 9. 13. 81 13. 85 14. 40 13. 40 1	9. 44 9. 54 9.	***************************************	0. 0843 . 0840 . 0848 . 0840 . 2508 . 2505 . 2505 . 2501 . 2589 . 2589 . 2586 . 2661 . 2586 . 2676 . 2686 . 2686 . 2742 . 2742 . 2742 . 2742 . 2823 . 2823 . 2823 . 2824 . 2824 . 2825 . 2826 . 2827 . 2827 . 2828 . 2828 . 2828 . 2828 . 2828 . 2828 . 2828 . 2828 . 2828 . 2829	0. 443-0 4360 4392 4410 3542 3553 3513 3780 3721 3781 3780 3794 3870 4154 4200 408 408 408 407 4154 4200 408 408 43815 3513 3545 3315 3555 3710 3800 4000 3745 4154 4200 4484 44557 4557 4553 4671 4680 4671 4724 4744 488 4344 4557 4557 4553 4671 4680 4671 4779 4781 4781 4782 4781 4782 4781 6711 6711 6711 6711 6711 6711 6711 6	1.50 1.55 1.57 1.57 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	83. 0 0 0 0 0 1 1 0 1 0 1 1 0 1 1 0 1 1 0 1 1 0 1 1 1 0 1 1 1 1 7 0 1 1 2 1 1 7 0 1 1 2 1 1 7 0 1 1 2 1 1 1 7 0 1 1 2 1 1 1 7 0 1 1 2 1 1 1 1 7 0 1 1 2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	9099HH9999119999009961191101H199911HH19HHH119HHH11911H19991999

TABLE XIII—Continued

[Stiffener pitch=2.50 inches. Stiffener depth, D=0.500 inch]

Sanai Ann	Walaha	Area	Failing	Failing	Thic	kness			Num-			Stiffener	Percent	m
Specifica- tion no.	Weight (pounds)	/cananen	load	stress (lb./sq.in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	ber of stiff- eners	Area of sheet	Area of stiff- eners	width (W- inches)	rein- force- ment	Type of failure
EB-485b EB-486a EB-486b EB-487b EB-487b EB-488b EB-489a EB-489b EB-490a EB-490a EB-491a EB-492b EB-492b EB-493b EB-493b EB-494b EB-495a EB-495a EB-496a EB-496a EB-497b EB-498b	2. 558 3. 823 3. 822 1. 306 1. 311 2. 626 1. 311 2. 626 1. 438 1. 405 1. 438 4. 238 4. 238 4. 238 4. 238 4. 238 4. 238 4. 248 4. 491 1. 451 1. 451 1. 451 1. 451 1. 551	1. 0040 1. 0020 1. 0650 1. 0650 1. 0650 1. 0650 1. 0650 1. 0620 1. 0620 1. 0680 1. 0680 1. 0680 1. 10680 1. 1160 1. 1160 1. 1160 1. 1160 1. 1160 1. 1160 1. 1163 1. 1163 1. 1163 1. 1163 1. 1163 1. 1173 1. 1173 1. 1175 1. 11775 1. 11775 1. 11775 1. 12485 1. 2485 1. 2485	99, 100 95, 600 85, 900 124, 800 124, 800 123, 900 124, 800 123, 900 123, 900 124, 600 114, 400 92, 100 105, 800 90, 600 92, 100 135, 500 122, 300 122, 300 122, 300 122, 300 123, 500 124, 600 125, 600 126, 600 127, 600 128, 600 129, 500 126, 600 127, 100 128, 600 129, 500 129, 500 126, 600 137, 100	111, 000 93, 500 85, 900 111, 600 93, 100 89, 800 80, 400 115, 900 134, 800 107, 400 99, 100 86, 300 121, 100 109, 000 121, 600 109, 300 92, 100 100, 700 97, 300 93, 300 101, 800	. 050 . 050	.031 .030 .030 .031 .031 .031 .031 .031	8. 90 13. 34 4. 25 4. 30 8. 60 8. 59 12. 89 12. 89 12. 89 12. 9. 24 9. 24 9. 24 13. 87 13. 88 4. 60 9. 20 9. 20 13. 83 13. 79 8. 86 8. 90 13. 33 4. 33 4. 30 4. 30 8. 4. 20 4. 4. 20	9. 19 9. 27 9. 27 9. 47 9. 57 9. 59 9. 50 9. 50 9. 56 8. 78 8. 78 8. 78 8. 78 8. 88 8. 89 8. 89 8. 89 9. 26 9. 22 9. 22 9. 22 9. 52 9. 52 9. 54 9. 55 9. 55	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	. 2865 . 2781 . 2781 . 2940 . 2965 . 2841 . 2945 . 2945 . 2945 . 4385 . 4385 . 4380 . 4390 . 4490 . 4490 . 4490 . 4490 . 4500 . 4500	. 7175 . 7239 . 7239 . 7730 . 7685 . 7849 . 7695 . 7705 . 7715 . 6225 . 6360 . 6360 . 6280 . 6280 . 6275 . 6750 . 6750 . 6750 . 6750 . 6730 . 6735 . 6730 . 6730	1. 25 1. 25 1. 25 1. 50 1. 50	69. 8 70. 7 71. 4 71. 2 71. 3 71. 3 71. 2 55. 5 54. 0 56. 5 55. 8 55. 8 55. 9 57. 1 57. 4 57. 1 57. 8 57. 2 59. 5 59. 5 60. 6	100011
EB-500a EB-500b EB-501a EB-501b	3. 072 3. 062 4. 600 4. 595	1. 2430 1. 2460 1. 2505 1. 2195	114, 100 117, 200 106, 200 100, 700	91, 800 94, 100 84, 900 80, 700	. 050 . 050 . 050 . 050	. 050 . 050 . 050 . 050	8. 64 8. 60 12. 87 12. 87	9. 49 9. 50 9. 49 9. 51	4 4 4	. 4745 . 4750 . 4745 . 4755	. 7685 . 7710 . 7760 . 7740	1. 50 1. 50 1. 50 1. 50	60. 5 60. 6 60. 8 60. 8	1 1 1

TABLE XIV

[Stiffener pitch=2.50 inches]

	<u> </u>	1	1	1	i ——				1		1			1
Specifica- tion no.	Weight (pounds)	Area (square inch)	Failing load (pounds)	Failing stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	Number of stiff- eners	Area of sheet	Area of stiff-eners	Stiffener width and depth (W and D inches)	Percent rein- force- ment	Type of failure
EC-502a EC-502b EC-503b EC-504b EC-505b EC-505b EC-506a EC-506a EC-507a EC-507a EC-507a EC-509b EC-510b EC-511b EC-511b EC-511a EC-512b EC-5154a EC-515b EC-515b EC-516b EC-516b EC-517b EC-517b EC-517b EC-517a EC-517b EC-517a	0. 418 422 845 882 1. 376 635 632 1. 246 1. 268 889 1. 901 2. 581 1. 717 2. 581 1. 717 2. 581 1. 282 1. 290 2. 609 2. 609 2. 609 2. 609 1. 512 2. 336 1. 094 1. 087 2. 178 2. 178 3. 613 1. 468 2. 953	0. 2125 2140 2143 2233 2322 2135 2435 2435 2400 2415 2400 2415 2670 2440 2685 2583 3380 3395 3380 3395 3380 3395 4180 4155 4180 4155 4160 4155 4160 4155 4580 4580 4580	11, 390 11, 970 10, 670 11, 600 11, 600 10, 880 10, 560 10, 560 10, 100 10, 300 10, 560 10, 560 10, 560 10, 560 10, 560 10, 560 10, 560 11, 500 11, 500 11, 500 11, 500 11, 500 18, 500 19, 500 19, 500 11, 500 11, 500 18, 570 19, 120 18, 12	53, 600 55, 900 40, 800 51, 950 46, 800 50, 850 43, 300 42, 100 42, 600 33, 500 35, 950 35, 950 35, 950 32, 100 34, 600 26, 400 51, 200 51, 200 51, 200 51, 200 52, 700 44, 600 44, 600 44, 600 42, 600 42, 700	0.009 .009 .009 .010 .010 .010 .009 .009	0.009 0.009 0.010 0.010 0.010 0.010 0.010 0.010 0.009	6. 88 6. 90 13. 78 20. 70 9. 12 9. 18 18. 15 18. 32 27. 44 12. 25 22. 69 22. 62 23. 62 23. 62 24. 47 27. 02 26. 98 40. 50 40. 48 9. 18 9.	8. 66 8. 64 8. 64 8. 64 8. 64 8. 64 8. 83 8. 65 8. 89 9. 11 9. 11 9. 12 9. 13 9. 33 9. 33 9. 38 8. 66 8. 66 8. 62 8. 89 9. 13 9. 13	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	0. 0780 0.0776 0.868 0.864 0.871 0.871 0.871 0.888 0.865 0.889 0.829 0.820 0.820 0.820 0.825 0.821 0.825 0.821 0.825 0.821 0.825 0.8	0. 1345 .1364 .1276 .1379 .1458 .1264 .1547 .1543 .1520 .1619 .1513 .1850 .1862 .1862 .1862 .1862 .1862 .1862 .1863 .1875 .2479 .2500 .2447 .2452 .2431 .1310 .1270 .1513 .1310 .1270 .1513 .171	0. 75 . 75 . 75 . 75 . 75 . 75 . 1. 00 1. 00 1. 00 1. 25 1. 25 1. 25 1. 50 1.	59. 9 60. 2 56. 1 58. 0 59. 3 55. 8 60. 7 60. 7 60. 4 64. 3 67. 3 60. 4 67. 3 61. 1 66. 1 66. 1 66. 1 71. 0 71. 1 71. 1 72. 29. 2 33. 3 32. 9 33. 2 35. 8 36. 9 37. 4 47. 4	HHAAAAFHHGHFACFCCCCCCCCCGGAAGEHHCCCCCHHHJ

TABLE XIV-Continued

[Stiffener pitch=2.50 inches]

					Thic	kness						Stiffener width		
Specifica- tion no.	Weight (pounds)	Area (square inch)	Failing load (pounds)	Failing stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	Number of stiff- eners	Area of sheet	Area of stiff- eners	and depth (W and D inches)	Percent rein- forced	Type of failure
EC_520h EC_521h EC_521a EC_521a EC_521a EC_521a EC_521a EC_521a EC_521a EC_521a EC_522b EC_522h EC_5252h EC_5252h EC_5552h	2. 965 4. 400 1. 930 3. 889 3. 889 1. 110 1. 112 2. 200 3. 347 1. 573 1. 563 4. 808 2. 140 4. 363 4. 808 2. 141 6. 445 6. 342 6. 211 6. 445 6. 342 6. 211 6. 445 6. 342 6. 211 6. 445 6. 342 6. 342 6. 343 6. 211 6. 344 6. 343 6. 211 6. 344 6. 343 6. 211 6. 344 6. 343 6. 211 6. 344 6. 343 6. 211 6. 344 6. 343 6.	0. 4560 . 4545 . 4520 . 5000 . 4965 . 5030 . 4965 . 5030 . 5025 . 5065 . 5640 . 5590 . 5650 . 5650 . 6100 . 6200 . 6110 . 6620 . 6110 . 6820 . 6110 . 6810 .	15, 220 17, 300 17, 360 17, 140 17, 900 17, 16, 850 17, 140 17, 16, 800 17, 17, 16, 800 17, 180 18, 000 23, 250 20, 180 22, 150 17, 900 23, 150 24, 140 20, 450 28, 520 28, 520 28, 520 28, 520 29, 830 30, 600 22, 950 22, 150 17, 900 23, 650 64, 400 24, 950 25, 670 26, 830 27, 700 28, 830 30, 600 29, 830 30, 600 21, 950 24, 440 27, 300 28, 830 30, 600 29, 830 21, 180 21, 180 22, 180 23, 650 64, 200 26, 830 27, 700 28, 830 29, 600 20, 830 21, 18	33, 400 38, 900 37, 600 34, 720 34, 500 35, 600 35, 600 35, 600 35, 600 35, 600 35, 600 36, 600 37, 600 38, 200 31, 100 36, 900 29, 900 41, 100 36, 900 46, 450 41, 500 41, 60	0.009 0.009	0.030 0.029 0.029 0.029 0.029 0.029 0.029 0.030 0.030 0.050	23. 40. 22. 40. 48. 40. 49. 11. 35. 22. 27. 50. 11. 34. 27. 50. 51. 51. 52. 51. 51. 51. 51. 51. 51. 51. 51. 51. 51	9.135 9.168	4 6 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	0. 2739	0. 1821 . 1890 . 1885 . 2265 . 2310 . 2230 . 2233 . 2285 . 2223 . 2285 . 1290 . 1330 . 1415 . 1860 . 1745 . 1860 . 1745 . 1860 . 1745 . 1860 . 1740 . 2170 . 2170 . 2180 . 2180 . 2190 . 4116 . 4195 . 4180 . 4195 . 4180 . 5049 . 5051 . 5100 . 5904 . 5068 . 5893 . 5909 . 7031 . 7030 . 7040 . 7050 .	1.25 1.25 1.50 1.50 1.50 1.50 1.50 1.50 1.50 1.5	39. 4 4 43. 5 5 4 6 6 1 2 2 4 4 3 9 9 9 4 4 2 2 4 4 3 3 3 4 4 4 4 4 4 2 2 2 4 5 3 3 3 1 4 6 6 2 2 2 4 5 3 3 5 5 5 6 3 5 7 5 5 8 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	CHHCCCCCGHHAHGHHHGCCCCICCGHHCCGGHGGGGGGGGGG

TABLE XIV—Continued

[Stiffener pitch = 2.50 inches]

					Thic	kness						Stiffener width	Percent	
Specifica- tion no.	Weight (pounds)	Area (square inch)	Failing load (pounds)	Failing stress (lb./sq. in.)	Stiff- ener (inch)	Sheet (inch)	Length (inches)	Width (inches)	Number of stiff- eners	Area of sheet	Area of stiff- eners	and depth (W and D inches)	rein- force- ment	Type of failure
EC-565a EC-565a EC-565a EC-565a EC-565b EC-566b EC-566b EC-566b EC-567b EC-567b EC-567b EC-577b EC-578b EC-578	4. 9479 4. 9479 7. 4722 7. 3913 3. 3599 6. 7725 10. 110 10. 12	0. 9465 9505 9400 1. 9401 1. 0410 1. 0430 1. 0410 1. 0410 1. 0410 1. 1650 1. 1670 1. 1700 1. 4290 1. 4290 1. 4290 1. 4290 1. 4290 1. 4290 1. 1200 9510 9583 9560 9560 9560 9560 1. 1220 911 1. 1160 1. 1260 1.	102, 550 106, 300 87, 400 87, 400 887, 400 887, 400 887, 400 887, 400 887, 400 887, 400 887, 400 887, 400 987, 500 998, 450	108, 300 112, 100 91, 900 93, 100 94, 900 86, 500 86, 400 88, 800 88, 200 88, 800 88, 200 120, 800 121, 300 120, 800 121, 300 120, 800 121, 300 120, 800 120, 800 121, 300 120, 800 120, 800 121, 300 120, 800 120, 800 121, 300 120, 800 121, 300 120, 800 121, 300 120, 800 121, 300 121, 300 122, 200 123, 800 124, 300 127, 800 127, 800 127, 800 128, 800 129, 800 120, 800 120, 800 121, 700 121, 300 122, 200 123, 800 124, 300 125, 700 126, 800 127, 300 127, 300 128, 800 129, 900 120, 800 121, 700 121, 300 122, 700 123, 800 124, 300 125, 700 127, 300 128, 800 129, 900 120, 800 121, 700 121, 300 122, 700 123, 300 124, 300 125, 700 127, 300 128, 800 129, 900 120, 800 121, 700 121, 300 122, 700 123, 300 124, 800 125, 500 127, 900 128, 800 129, 900 120, 900 121, 900 121, 900 122, 900 123, 300 124, 900 125, 500 127, 900 128, 900 129, 900 120, 900 121, 900 121, 900 122, 900 123, 300 124, 900 125, 900 127, 900 128, 900 128, 900 129, 900 120	0.030 0.030 0.030 0.030 0.030 0.030 0.030 0.030 0.030 0.030 0.030 0.031 0.030	0.050 0.049 0.049	18. 47 18. 47 27. 49 27. 49 27. 49 27. 49 21. 13. 21. 70 22. 70 23. 4. 00 24. 0. 49 25. 70 26. 89 27. 00 26. 89 27. 00 20. 66 20. 66 20. 66 21. 70 20. 66 20. 66	8.95 8.95 9.114 9.914 9.	******************	0. 4485	0. 4855	1.00 1.00 1.00 1.00 1.00 1.00 1.25 1.25 1.25 1.25 1.50 1.50 1.50 1.50 1.50 1.50 1.50 1.5	90.27.96.13.30.07.2.28.88.89.01.13.00.27.89.28.88.89.01.22.28.88.89.01.22.28.88.89.01.22.28.88.89.01.22.28.88.89.01.22.28.88.89.01.22.28.88.89.01.22.28.88.89.01.22.28.89.01.30.02.79.01.30.00.27.77.77.77.77.77.77.77.77.77.77.77.77.	140011

TABLE XV

Specifi-	Weight	Area	Failing	Failing	Thick	rness	Length	Width	Number	Area	Area	Stiff- ener	Percent rein-	Type of
cation no.	(inches)	(square inch)	load (pounds)	stress (lb./sq. in.)	Stiffener (inch)	Sheet (inch)	(inches)	(inches)	of stiff- eners	of sheet	of stiff- eners	depth (D in- ches)	force- ment	failure
F-609a F-609b F-610a F-6110a F-6111a F-6112b F-612b F-612b F-613b F-613b F-614b F-615a F-616a F-6167a F-617a F-617a F-6181b F-6182b	0. 590 . 594 1. 352 1. 360 2. 358 2. 232 1. 010 1. 000 2. 228 3. 541 3. 583 1. 467 1. 446 4. 821 1. 591 1. 591 1. 591 595 505 505 505 505 505 505 505	0. 2275	14, 260 14, 325 16, 200 15, 400 13, 600 24, 150 24, 770 25, 020 20, 930 19, 920 38, 480 38, 610 24, 770 20, 740 40, 700 41, 150 41, 15	62, 700 63, 000 61, 750 58, 700 45, 400 66, 650 62, 500 57, 200 41, 200 67, 800 69, 000 38, 700 40, 250 42, 900 66, 650 80, 250 61, 300 65, 400 65, 500	0.010 010 009 009 009 010 010 009 009 009	0. 010 010 010 010 010 010 030 030 031 031	9. 06 9. 10 18. 05 18. 15 26. 04 26. 01 9. 06 9. 06 18. 02 18. 05 26. 08 25. 99 9. 04 9. 04 9. 04 9. 06 9. 07 18. 04 26. 03 26. 04 9. 07 18. 04 26. 03 26. 04	8. 50 8. 59 8. 59 8. 49 8. 49 8. 47 8. 50 8. 44 8. 53 8. 43 8. 53 8. 43 8. 43 8. 44 8. 53 8. 44 8. 53 8. 44 8. 53	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	0. 0850 0. 0847 0.859 0.858 0.849 0.849 2.2541 2.252 2.2618 2.2628 4.150 4.150 4.150 4.350 4.	0.1425 .1433 .1761 .1762 .2311 .2149 .1359 .1310 .1708 .1712 .2112 .212 .2180 .1540 .2160	0. 50 . 50 1. 00 1. 50 1. 50 . 50 1. 00 1. 50 1.	58. 7 58. 9 63. 8 69. 8 68. 2 30. 4 35. 5 40. 5 40. 5 40. 5 30. 2 22. 8 22. 8 23. 2 24. 3 25. 6 26. 8 26. 8 27. 8 28. 8 29. 2 29. 2 29. 3 29. 3 20. 3	GGG J I I A A I I I I H H I I I I H H I I I

Table XVI—Corrugated stainless steel specimens

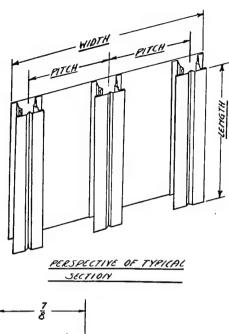
Specifi-		Area	Failing	Failing	Thie	kness	Length	Width	Number	Area of sheet	Area corru-	Pitch	• .	R/t
cation no.	Weight (pounds)	(square inch)	load	stress (lb./sq. in.)	Stiffener (inch)	Sheet (inch)	(inches)	Width (inches)	of corru- gations	(square inch)	gated (square inch)	(inches)	Llø	R/t
T-1a	0, 144	0. 1215	5,000	41, 150	0.010		4.0	8. 563	8		0. 1215	1.00	22.8 22.8	25 25 25 25 25 25 26 27 25 20 25
I-1a I-1b I-2a I-2b	. 139	. 1258	7,700	61, 200	.010		4.0	8. 563	8		.1258 .1170	1.00	22.8	25
I-2a	.139 .268 .268	. 1170	9, 350	79, 950	.010		8.0	8.469	8		.1170	1.00	45.6	25
Į-2b	. 268	.1170	7,900	67, 500	.010		12.0	8.50	8		. 1165	1.00 1.00 1.00	68. 4	25
I–3a I–3b		.1258 .1170 .1170 .1165 .1195 .1048 .1092	11,000	92,000	.010 .010 .010 .010 .010		4. 0 8. 0 8. 0 12. 0 12. 0	8. 469 8. 469 8. 50 8. 50 9. 50 9. 50	8 8 8 8 8 8		.1165 .1195 .1048 .1092 .1092 .1092 .1131 .1090 .0875 .0875 .0874 .0875 .2430 .2335 .2405	1.00	45. 6 45. 6 68. 4 68. 4 22. 35 22. 35 44. 7 44. 7 67. 1	25
I-4a	120	. 1048	9, 330	89,000	.010		4.0 8.0 8.0 12.0 4.0 4.0 8.0 12.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 8.0 8.0 8.0 12.0 4.0 4.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8.0 8	9. 50	6		. 1048	1.50	22. 35	40.5
I-4a I-4b	. 125	- 1092	4, 500	41, 200	. 010		4.0	9.50	6		1002	1.50	44.7	40. 5
I-5a I-5b	. 250	. 1092	6,000	55,000	.010		8.0	9. 625	6		1092	1.50	44.7	40.5
1-5D	. 250	.1092 .1092 .1131 .1090	8 400	56 600	010		12.0	9, 594	6		. 1131	1.50	67. 1	40. 5 40. 5
I-6a I-6b	374	1090	7, 800	71, 500	. 010		12.0	9. 594	6		. 1090	1.50	67. 1	40.5
I–7a I–7b	.100	. 0875 . 0875 . 0874	4, 330	49, 500	.010 .010 .010 .010 .010 .010 .010		4.0	9, 625 9, 625 9, 594 9, 594 8, 50 8, 625 8, 625 8, 594 8, 469 8, 469 8, 469 8, 469	4		. 0875	2.00	92 10	66 66 66
I-7b	.100	. 0875	3,750	42,900	.010		4.0	8.00	4		0874	2.00	44. 38	66
I-8a I-8b	.200	. 0874	4,450	1 50,900	010		8.0	8. 625	4		.0874	2.00	44.38	66
I-00	300	0875	3, 500	40,000	.010		12.0	8. 594	4		. 0875	2,00	66. 55	66
I-9b	.300	.0875	3,600	41, 100	. 010		12.0	8. 594	4		. 0875	2.00	90.55	66 12. 5 12. 5
I-9a I-9b I-10a I-10b	. 278	.0875 .0875 .2430 .2335 .2405	36, 300	149, 300	.020		4.0	8.469	8 8 8 8 8 8		2430	1.00	22.8	12. 5
<u>I</u> -10b	. 267	. 2335	33,650	149, 100	020		8.0	8. 469	8		2405	1.00	45.6	12.5
1-118 1-11b	550	2405	27 100	112, 700	.020		8.0	8. 469	8		. 2405	1.00	45.6	12. 5
I-12a	. 863	. 2515	28,000	111, 300	. 020		12.0	8. 531	8	1	1 9515	1.00	68.4	12.0
I-12b	. 826	. 2515 . 2405 . 2230 . 2230	33, 400	138, 800	.020		12.0	8. 531 8. 531 9. 531 9. 531 9. 469 9. 531 9. 531 9. 531	8 6		. 2515 . 2405 . 2230 . 2230	1.00	92.35	20.3
I-13a	. 255	. 2230	29,900	134,000	020		4.0	9. 531	6		2230	1.50	22. 35	20. 3
1-13D	. 255	2230	26 200	111 300	. 020		8.0	9. 469	6		. 2350 . 2350	1.50	44.7	20. 3
I-14b	538	2350	29, 650	126, 100	. 020		8.0	9.469	6		. 2350	1.50	44.7	20.3
I-15a	. 785	. 2250 . 2350 . 2350 . 2290 . 2315 . 1860	28, 400	124, 000	.020		12. 0 12. 0 4. 0 4. 0	9. 531	6		. 2290 . 2315 . 1860	1.50	67.1	20.3
I-15b	. 795	. 2315	28, 200	121,700	.020		12.0	8. 469	0		1860	2.00	22, 19	33
I-16a	.213	. 1860	18,500	99,500	020		4.0	8, 469	4		1860	2.00	22, 19	33
I-17a	435	. 1860 . 1900 . 1900 . 1923 . 1923	19, 450	102, 300	.020		1 8 0	8. 469 8. 469	4		. 1860 . 1900	2.00	44. 38	33
I-17b	435	1900	19,800	104, 200	. 020		8.0	8. 469	4		. 1900 . 1923	2.00	88 55	33
I-18a	. 660	. 1923	19,400	100, 900	.020		12.0	8.094	3		1923	2.00	66, 55	33
I-18b	. 660	. 1923	18, 400	195, 500	030		4.0	8, 531	8		. 1923 . 3800	1.00	22.8	8.
1-19a 1-19b	435	3800	67, 700	196, 000	,030		4.0	8, 531	8		. 3800	1.00	22.8	8.
Î-20a	855	3735	69, 700	160,000	. 030		8. 0 12. 0 12. 0 4. 0 4. 0 8. 0	8. 594 8. 594 8. 531 8. 531 8. 50	8		. 3735 . 3835 . 3980 . 3970	1.00	67. 1 22. 19 44. 38 44. 38 66. 55 66. 55 22. 8 45. 6 45. 6 45. 6 44. 7 44. 7 44. 38 46. 55 22. 35 44. 7 44. 38 46. 55 22. 35 44. 7 44. 38 46. 55 46. 55 46. 55 46. 55 46. 55 46. 55 46. 55 46. 6 45. 6 46. 55 46. 55	8.
I-20b	. 877	. 3735 . 3835	24,000	64, 300	.030				8		3080	1.00	80 4	8.
I-21a	1.366	.3980 .3970	50, 100	125, 800	. 030		12.0	8, 531	8		3970	1.00	68. 4	8.
I-210	384	.3440	57 500	167, 000	.030		4.0	9. 50	6		. 3440 . 3440	1.50	22. 35	13.
I-22b	. 384 . 384 . 795 . 758 1. 168 1. 145	.3440	5,000 7,700 9,350 7,900 10,400 11,000 11,000 6,000 6,250 6,400 7,800 4,330 3,500 3,500 3,500 33,650 3,500 33,400 27,100 28,000 28,200	41, 150 61, 200 79, 950 67, 500 92, 000 89, 000 41, 200 55, 600 57, 250 49, 500 41, 100 41, 100 41, 100 149, 300 141, 100 111, 300 111, 300 121, 700 121, 700 135, 800 136, 800 137, 800 138, 800 138, 800 139, 500 99, 500 99, 500 102, 300 104, 200 109, 500 101, 300 101, 300 102, 300 105, 500 106, 000 107, 800 107, 800 107, 800 108, 300 117, 800 117, 800 117, 800 117, 800 118, 300 118, 300 118, 300 119, 300 119, 300 110, 300 110, 300 110, 300 111, 300 111, 300 112, 800 114, 300 115, 800 117, 800 117, 800 118, 300 119, 300 119, 300 110, 300 111, 300 111, 300 111, 300 112, 300 113, 300 114, 300 115, 300 116, 300 117, 800 117, 800 118, 300 119, 300 119, 300 119, 300 110, 300 110, 300 110, 300 110, 300 111, 300 111, 300 112, 300 113, 300 114, 500 115, 300	. 020 . 030 . 030		12.0 12.0 4.0 4.0 8.0	8. 531 8. 531 9. 50 9. 50 9. 50 9. 50 9. 50	6		. 3440	1.00 1.50 1.50 1.50 1.50 1.50 1.50 1.50	68. 4 22. 35 22. 35 44. 7 44. 7	12. 5 12. 5 12. 5 20. 3 20. 3 20. 3 33 33 33 33 8. 8 8. 8 8. 8 13. 4
I-23a	. 795	3475	53, 700	154, 600	. 030		8. 0 8. 0	9.50	6			1.50	44.7	13.
I-23b	. 758	.3310 .3400	18, 600	56, 200	. 030		12.0	9, 50	8		3400	1.50		13.
I-11a I-11b I-12a I-12b I-12b I-13b I-14b I-15b I-16a I-16a I-16a I-17a I-17a I-17a I-17a I-17a I-12a I-12a I-12b I-20a I-21a I-22a I-22a I-22a I-22a I-22b I-22a	1.168	3340	42,000	78, 100	.030			9. 50	6		. 3400 . 3340 . 2980	1.50	67.1	13.
I-25a	341	. 3340 . 2980 . 3162	45, 500	152, 100	.030		4. 0 4. 0 8. 0 8. 0	9. 50 9. 50 8. 50 8. 50 8. 50 8. 50 8. 50	4		. 2980	2.00	67. 1 22. 19 22. 19 44. 38 44. 38 66. 55 66. 55	22
I-25b	.362	.3162	50,600	160,000	. 030		4.0	8. 50	4		. 3162 . 2930	2.00	44 39	22
I-26a	. 670	. 2930 . 2930	41, 450	141, 500	. 030		8.0	8.50	1 .4		2930	2.00	44, 38	22
I-25a I-25b I-26a I-26b I-27a I-27b	.341 .362 .670 .670 .950	. 2930	41,900	143,000	. 030		12.0 12.0	8.50	4		2770	2.00	66. 55	22 22 22 22 22 22
1-2/8	. 950	. 2770 . 2770	32, 200	110, 500	.000		12.0	9 50	1 4	1	. 2770	2.00	66, 55	22

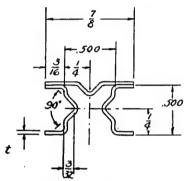
¹ Tested on knife-edges.

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Table XVI—Corrugated stainless steel specimens—Continued

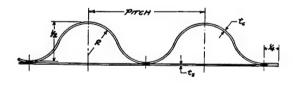
Specifi-	NV-1-ha	Area	Failing		Thickness		Length	Width	Number	Area of sheet	Area corru-	Pitch	7/0	R/t
cation no.	Weight (pounds)	(square inch)	load (pounds)	stress (lb./sq. in.)	Stiffener (inch)	Sheet (inch)	(inches)	(inches)	of corru- gations	(square inch)	(square inch)	(inches)	<i>L</i> /ρ	
I-28a	. 329	. 2881	40, 570	140, 700	. 019	0.005	3. 99	8.47	8	0. 0423	. 2458	1.00	21. 4 21. 4	
I-28b	.340	. 2987	43, 680	146, 400	.019	, 005 , 005	3. 98 8. 0	8. 54 8. 50	8	. 0426	. 2561 . 2490	1.00 1.00	43.0	
I-29a I-29b	. 667	. 2915	36, 200 40, 500	124, 200 139, 700	. 019	. 005	7, 99	8. 52	8	. 0426	. 2474	1.00	42.9	
I-30a	1.027	.3000	43,075	110, 250	. 019	. 005	11.97	8.48	200000000000000000000000000000000000000	. 0424	. 2576	1.00	69. 8 69. 9	
I-30b	1.009	. 2940	32, 050	109, 000 132, 000	.020 .019	. 005 . 010	11. 99 3. 97	8. 52 8. 52	8	. 0426	. 2514	1.00 1.00	21.0	
I-31a I-31b	.372	. 3275 . 3328	43, 210 46, 930	140, 900	020	.010	3.98	8.50	8	. 0850	. 2478	1.00	21.0	
I-32a	.741	, 3243	42, 330	130, 400	.019	. 010	7, 98	8. 58	8	. 0858	. 2385	1.00	42.1 42.2	
I-32b	. 752	. 3290	43, 190	131, 200	.019	. 010	7. 99 11. 94	8. 58 8. 55	8	. 0858	. 2432	1.00 1.00	63. 1	
I-33a I-33b	1. 152 1. 126	.3372	33, 990 34, 410	100, 900 104, 600	.019	.010 .010	11.97	8.48	8	.0848	. 2440	1.00	63. 3	
I-34a	. 415	.3643	46, 990	129, 000	. 019	. 014	3, 98	8. 52	8	. 1193	. 2450	1.00	21. 2	
I-34b	. 420	. 3690	44, 560	120, 900	.019	. 015	3.98	8, 49	8	. 1272 . 1187	. 2418	1.00 1.00	21. 2 42. 5	
I-35a	. 830	.3626	42, 610 43, 180	117, 500 117, 600	.019	.014	8. 0 7. 98	8. 48 8. 50	8	1275	2395	1.09	42.4	
I-35b I-36a	. 838 1, 285	.3670	34, 590	92, 300	.019	.015	11.99	8. 50	8	. 1275	. 2471	1.09 1.00	63. 6	
I-36b	1. 278	. 3730	36, 560	92, 300 98, 000	.019	. 014	11.98	8. 52	8	. 1192	. 2538 . 2415	1.00	63. 6 21. 4	
I-37a	. 471	. 4115	43, 400	105, 200	.020	.020	4, 40	8. 50 8. 50	8	.1700	2300	1.00	21.4	
I-37b	. 458 . 926	. 4000 . 4050	43, 900 46, 980	109, 800 116, 000	.020	.019	7. 99	8. 51	8	.1618	. 2432	1.00	42.7	
I-38a I-38b	916	4000	40, 200	100, 500	, 020	. 020	8.0	8. 50	8	. 1700	. 2300	1.00	42.7	
I-39a	1.412	. 4121	32, 920.	79, 800	.019	. 019	11.97	8, 50	8	. 1615 . 1613	. 2506 . 2491	1.00 1.00	64. 0 64. 0	
I-39b	1.406	. 4104	38, 480 59, 230	93, 700 118, 300	.019	.019	11.98 3.98	8. 49 8. 55	8	2480	2533	1.00	21.8	
I-40a I-40b	. 571	. 5013	59, 450	120, 500	.019	. 029	4.0	8.49	8	. 2460	. 2470	1.00	21. 9	
I-41a	1, 130	. 4947	57, 790	120, 500 117, 000	.019	. 029	7.98	8. 53	8	. 2474	. 2473	1.00	43.7 43.7	
I-41b	1. 100	. 4815	56, 250	116, 900 88, 400 91, 300	.019	. 029	7.98 11.98	8.49 8.50	8 8	. 2460 . 2465	. 2355	1,00 1,00	65.6	
I-42a I-42b	1.675	. 4888 . 4823	43, 200 44, 050	98, 400	.019	. 029	11.97	8.49	8	2460	. 2363 1905	1.00	65, 6	
I-43a	1. 652 . 267	233	26, 000	1 111.600	.020	. 005	4.0	8.50	4	. 0425	. 1905	2.00	21. 1 21. 1	
I-43b	. 267	. 233	25, 200	108, 200 110, 400	. 020	. 005	4.0	8.50	1 1	. 0425	. 1905 1951	2.00	42.1	
I-44a	. 544	. 238	26, 250 24, 800	110, 400	.020	.005	7.98 7.98	8. 58 8. 58	1	0429	1906	2.00	42.1	
I-44b I-45a	. 533	2385	23 600	08,000	.020	005	11.98	8. 58	4	. 0429	. 1956	2.00 2.00 2.00	63. 2	
I-45b	.812	. 2365	21,900	92, 600 81, 500 92, 400	.020	.005	12. 0 3. 97	8. 57	4	. 0429	. 1936 . 1790	2,00	63. 2 20. 8 20. 9	
I-46a	. 300	. 264	21,500	81,500	.020	. 010 . 010	3.97	8. 50 8. 52	1	.0852	. 1798	2 00	20.9	
I-46b I-47a	. 302	. 265	24, 500 25, 400	92, 700	.020	.010	7, 98	8, 55	4	. 0855	. 1885	2,00	41.8	
I-47b	. 625	. 274 274	25, 400 27, 200 27, 500	92, 700 99, 300	.020	.010	7.97	8, 55	1 4	. 0855	. 1885	2,00 2,00 2,00 2,00	41. 8 62. 9	
I-48a	. 945	2755	27, 500	99, 900 96, 200 90, 800	.020 .020	.010 .010	12.0 12.0	8. 61 8. 54	1	. 0854	1896	1 2.00	62.9	
I-48b I-49a	.945	. 2750 . 322	26, 500 29, 220	90, 800	.019	.015	3, 98	8. 52	4	. 1278	, 1942	2.00	21. 2	
I-49b	. 366	. 322	31, 020	1 96.3(1)	.020	.015	3. 97	8. 52	4	. 1278 . 1273	. 1942	2.00 2.00	21. 2 42. 5	
I-50a	. 721	. 3154	27, 760	87, 900 86, 300	.020	.015	7. 99 7. 97	8. 49 8. 50	4	1274	. 1896	2.00	42.4	
I-50a I-51a	1. 073	. 3170	27, 360 20, 770	66, 400	.019	.015	11.99	8 52	1 4	, 1278	, 1852	2 00	63.8	
I-51b	1. 075	. 3138	21, 150	67, 400	. 020	.015	11.98	8, 64	4	. 1295	. 1843	2.00	63. 8 21. 2	
I-52a	. 392	. 3478	29, 440	84, 700	. 020	.019	3. 94 3. 96	8, 64 8, 53 8, 53	1 1	. 1279	. 2199	2.00	21. 2	
I-52b I-53a	. 397	. 3455 . 346	30, 190 23, 650	87, 400 68, 400	.020	.019	8.0	8.52	1 4	. 1278	. 2182	2.00	43.1	
I-53b	. 799	. 350	27 000	77 200	.019	. 019	7, 98	8, 51	4	. 1277	, 2223	2.00	43. 0 64. 4	
I-54a	1. 182	. 3455	23, 030 22, 110	66, 700 63, 000 78, 600	.019	.019	11. 96 11. 96	8. 46 8. 51	4	. 1610 . 1617	. 1845	2.00	64. 4	
I-54b	1, 202 , 491	. 3511	22, 110 33, 780	78 800	.019	.019	3. 99	8. 56	4	. 2570	. 1730	2,00	22. 4	
I-55a I-55b	. 495	4345	32, 040	73, 800	. 020	. 029	3. 98	8. 51	4	. 2465	. 1880	2.00	22. 4	
I-56a	, 990	, 4330	31,020	71, 700 85, 700	. 019	. 029	7.99	8. 57	4	. 2485 . 2460	. 1845 . 1916	2.00	44.9	
I-56b	. 993 1, 486	4376	37, 460 22, 980	85, 700 53, 000	.019	029	8. 0 11. 99	8. 50 8. 52	1 4	2470	. 1865	2,00	67. 4	
I-57a I-57b	1, 486	4330	25, 680	59, 300	.019	.029	12.0	8, 52	4	. 2470		2.00	67. 5	
1-010	1 2, 200	1		1	1	1	1	1	1			1	1	1





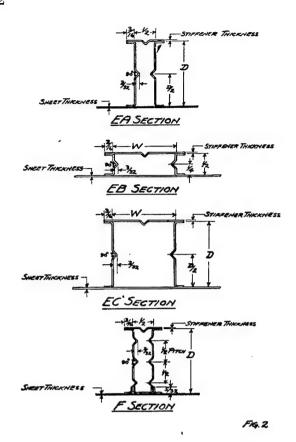
STIFFENCE SECTION
A.6 C SERES
FIG.-1

CORRUGATED STAINLESS STEEL I- SERJES

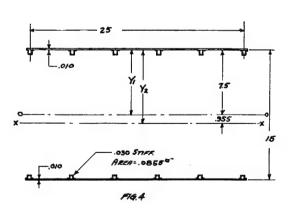


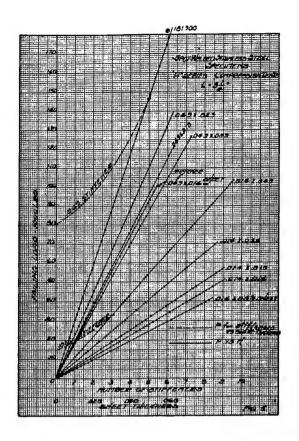
PITCH IN.	R IN.	RADIUS OF GYRATION S
.00	.250	.1754
1.50	.405	./789
2.00	.660	./803

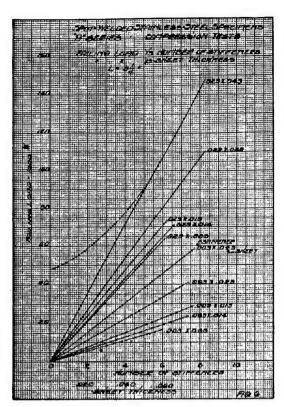
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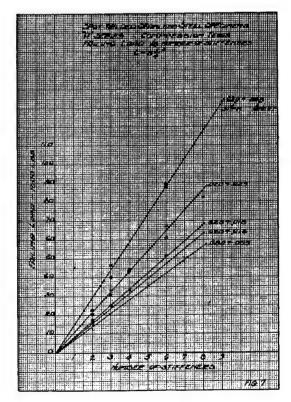


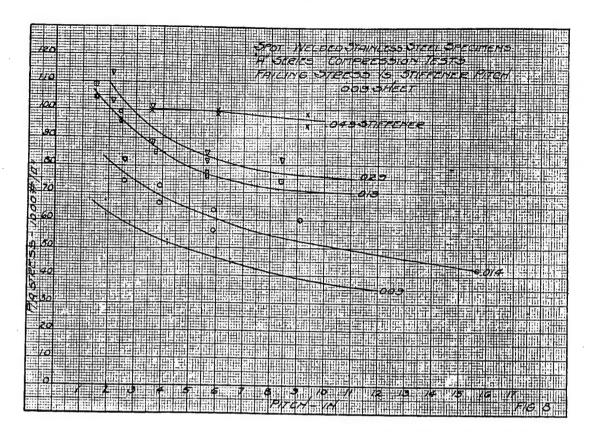
SIMPLE BOX BEAM

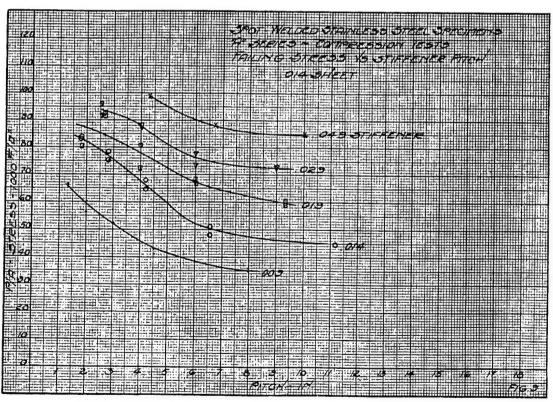


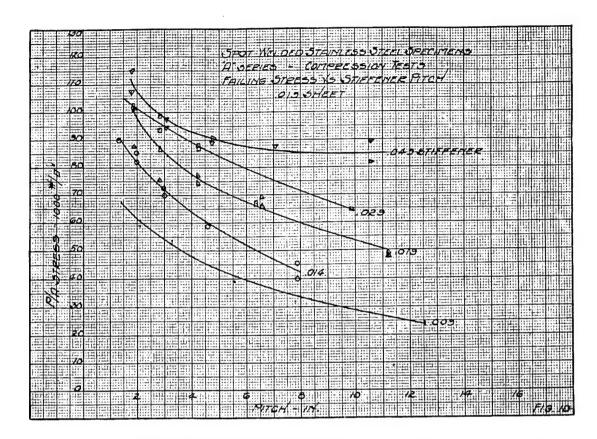


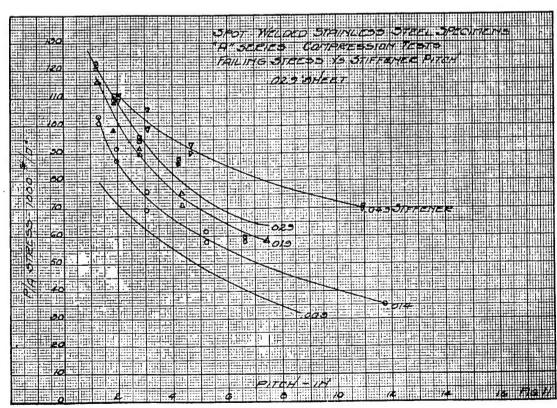


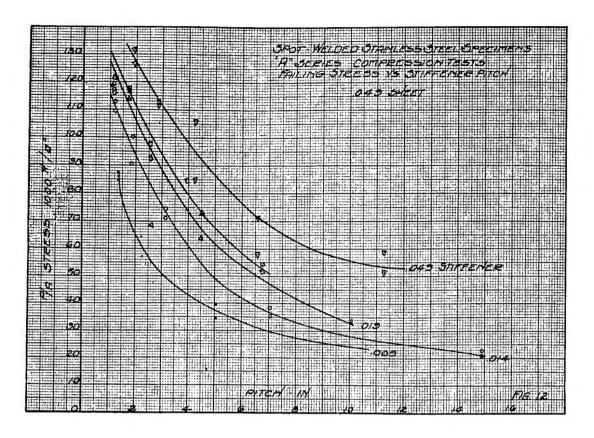


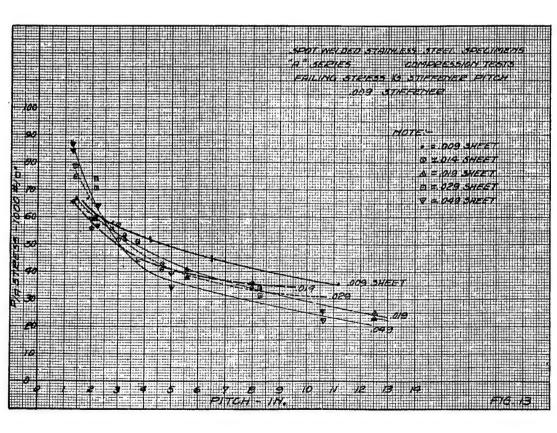


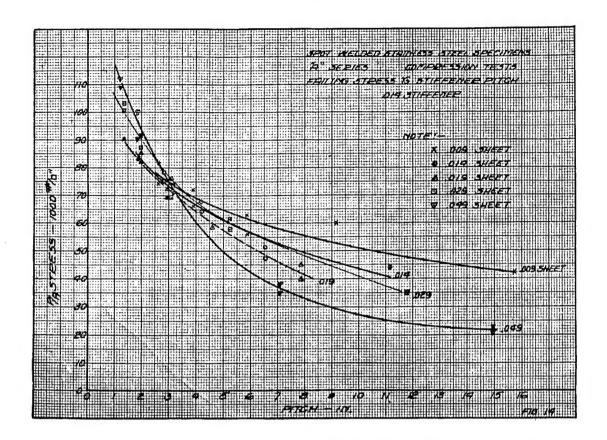


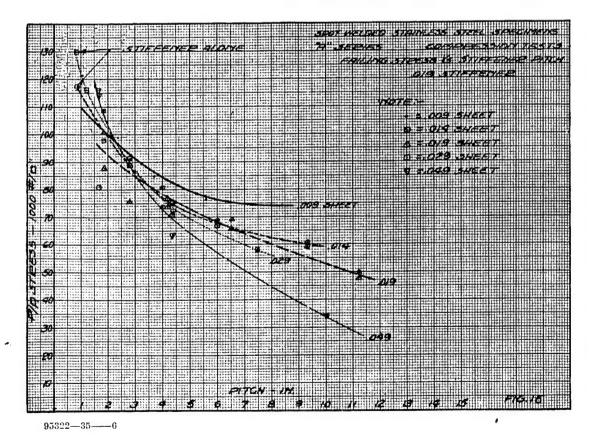


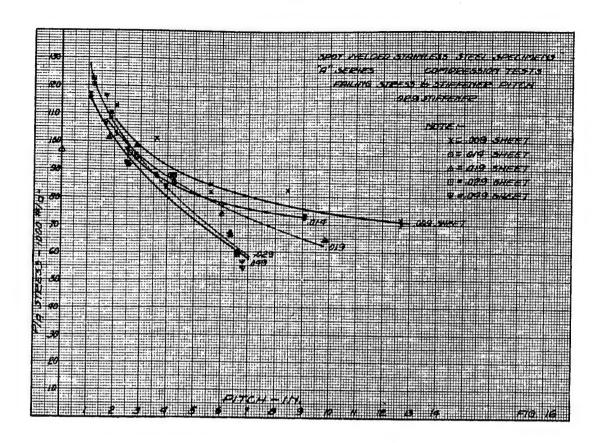


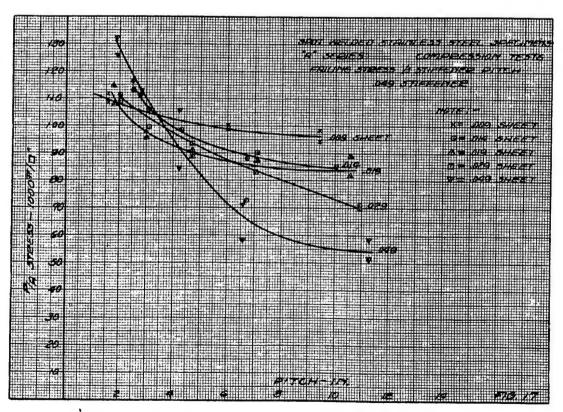


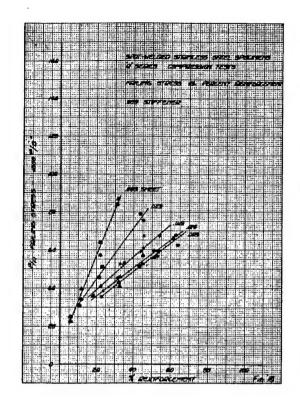


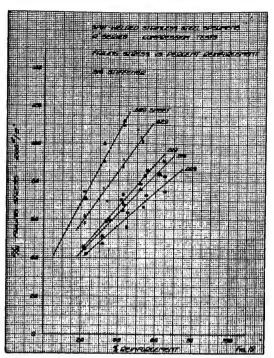


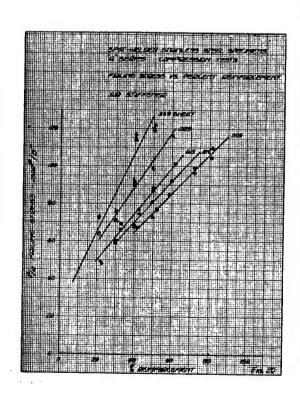


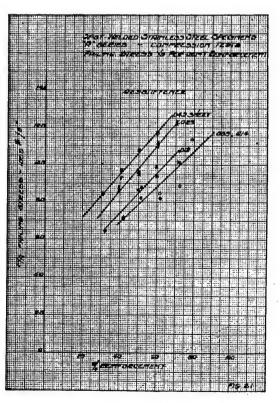


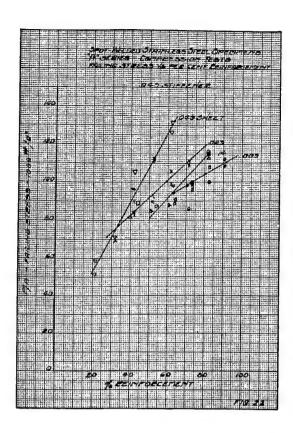


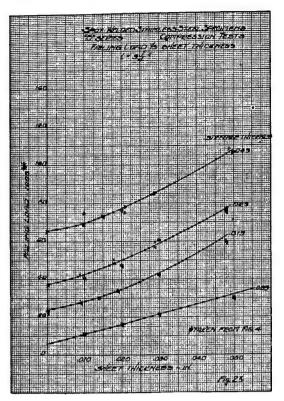


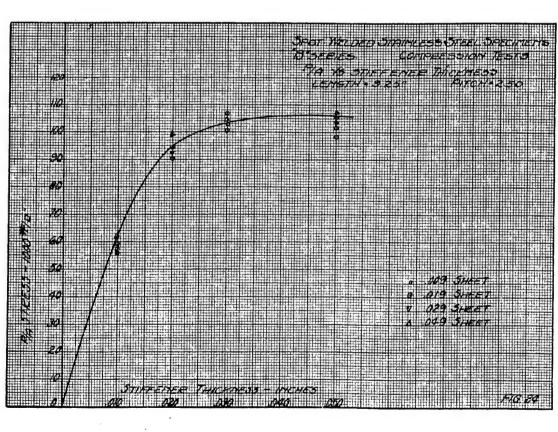






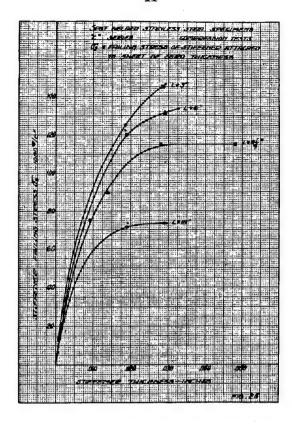


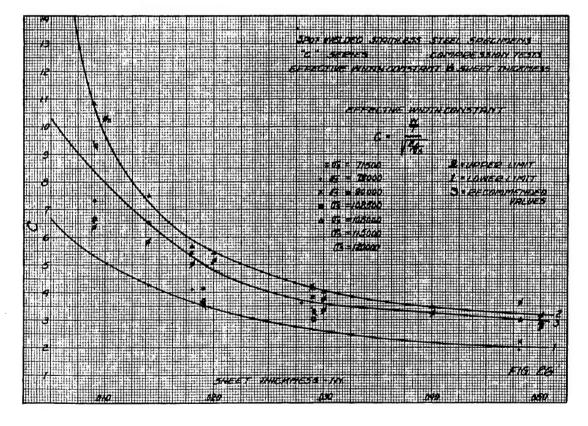


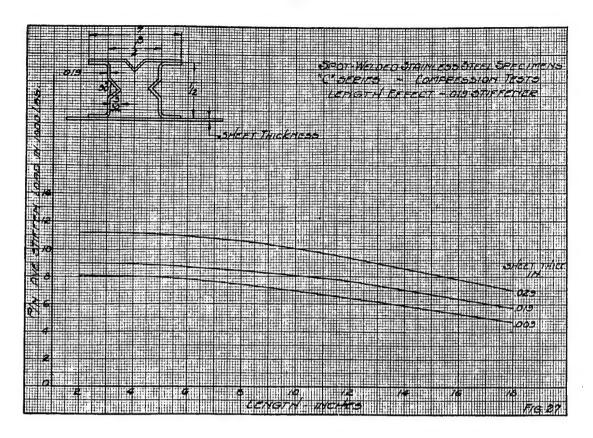


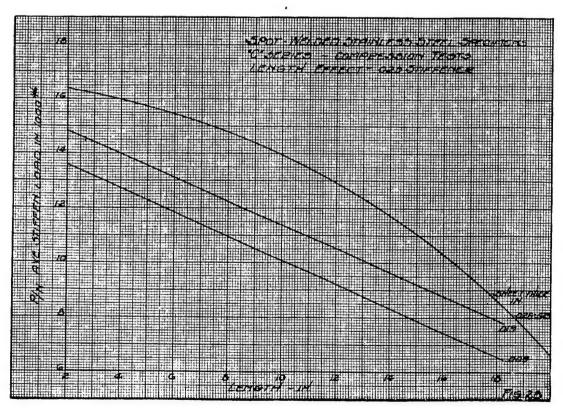
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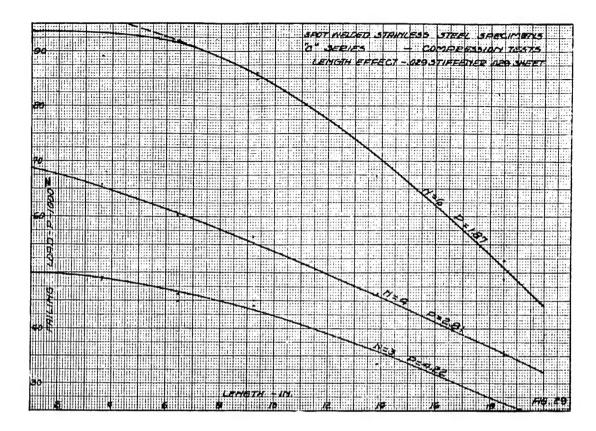
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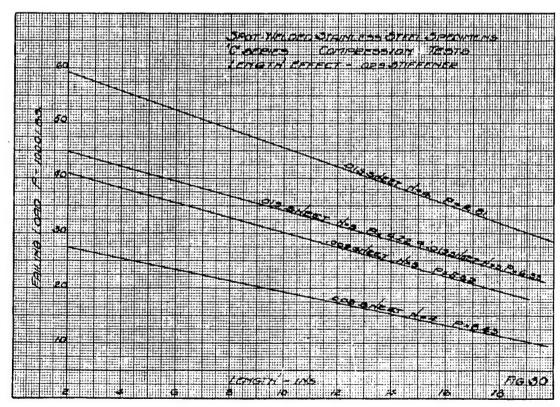


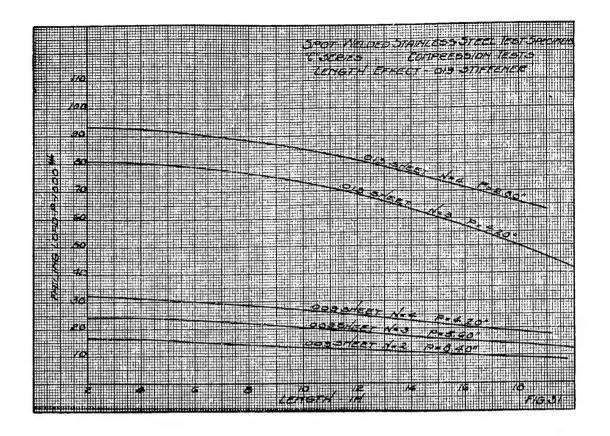


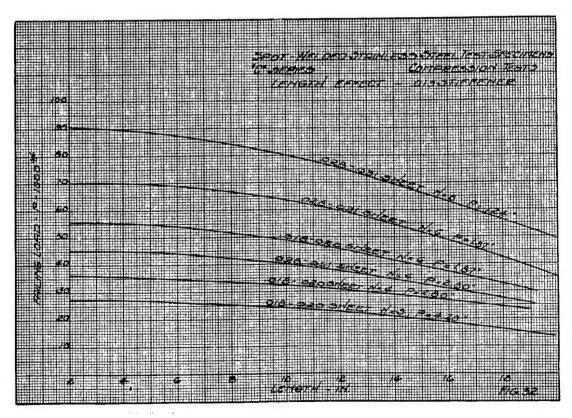




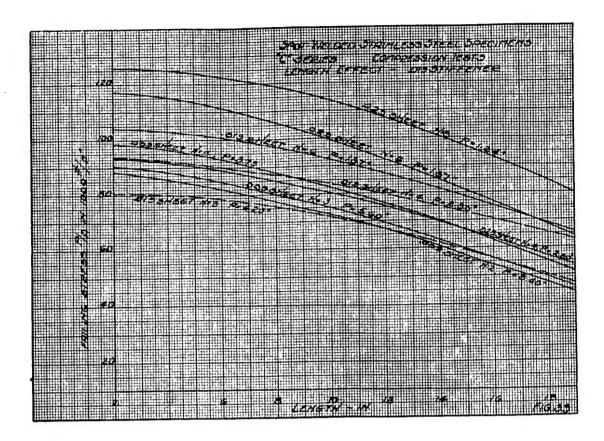


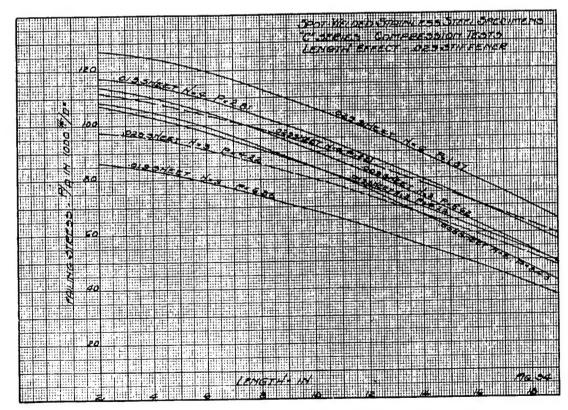


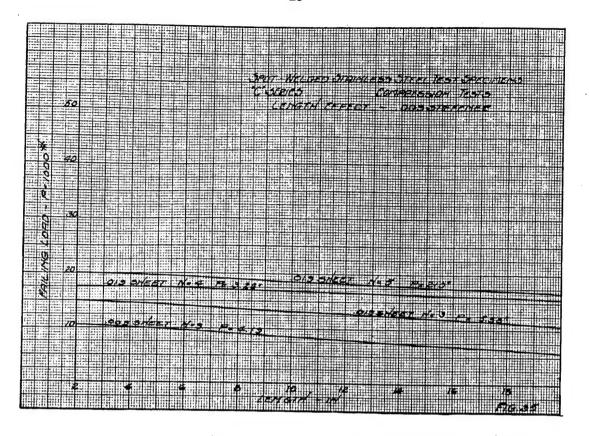


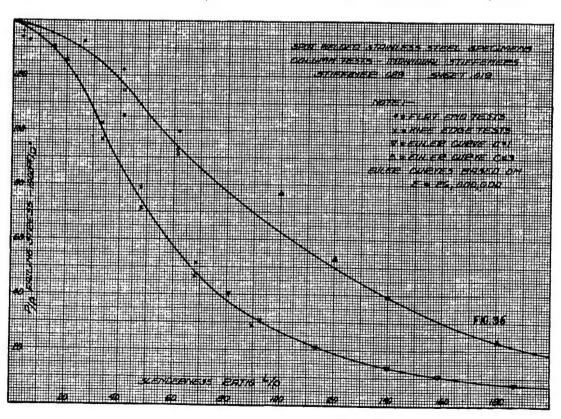


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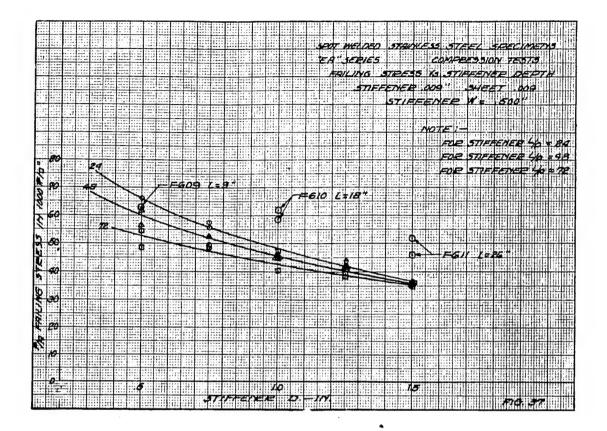


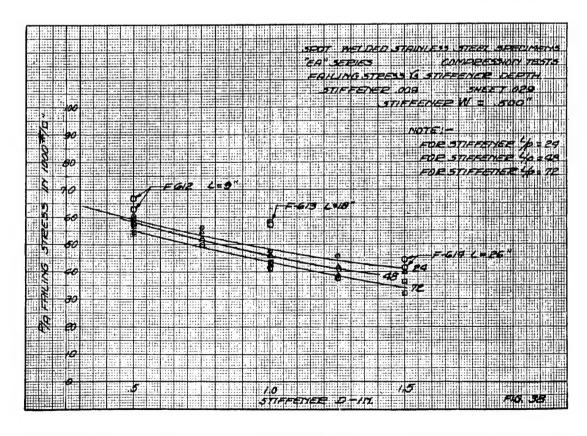


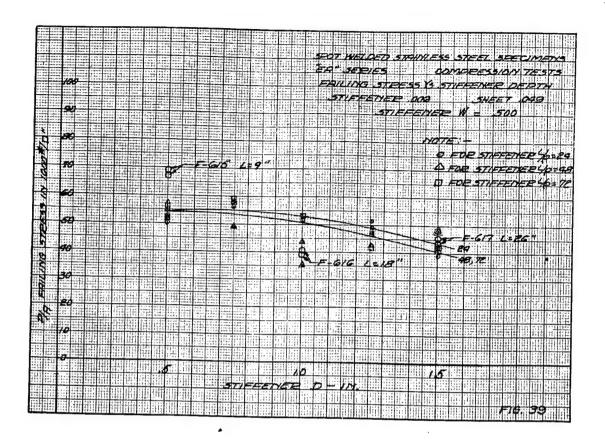


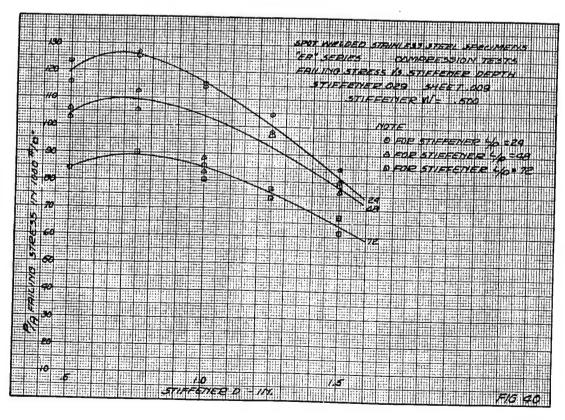
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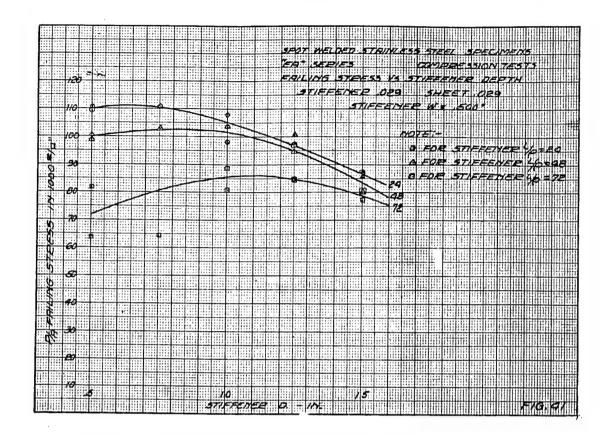
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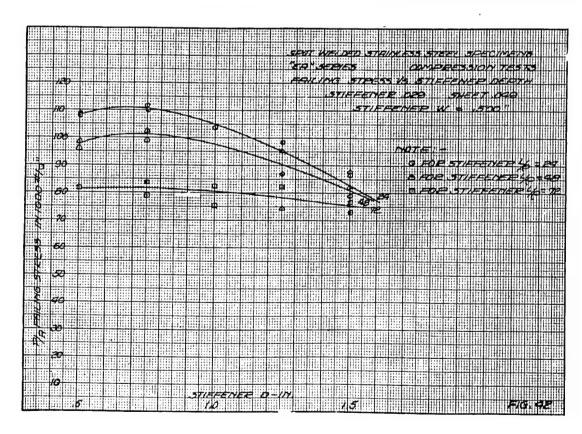


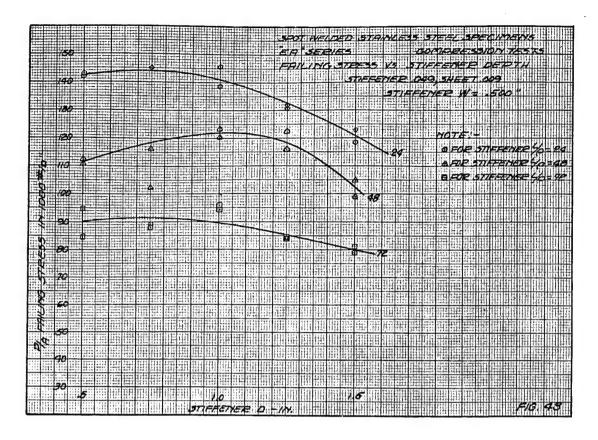


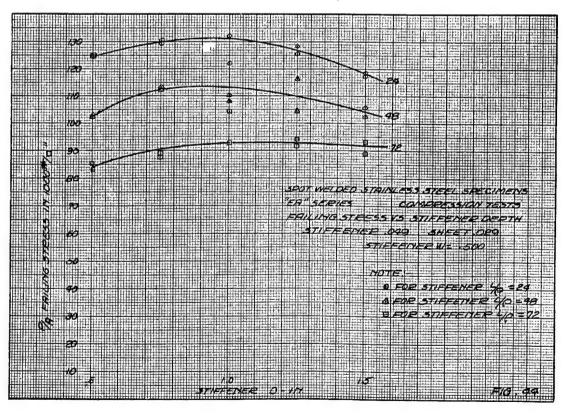


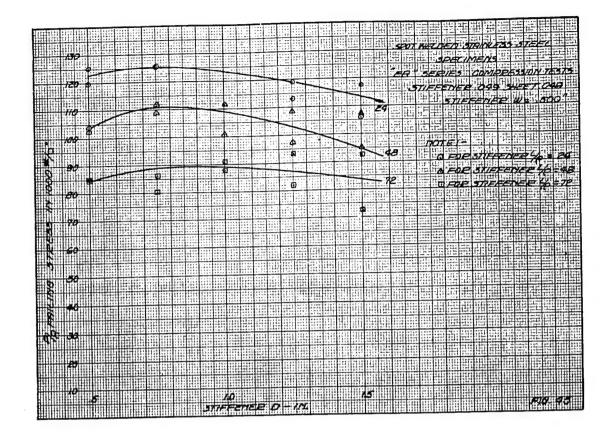


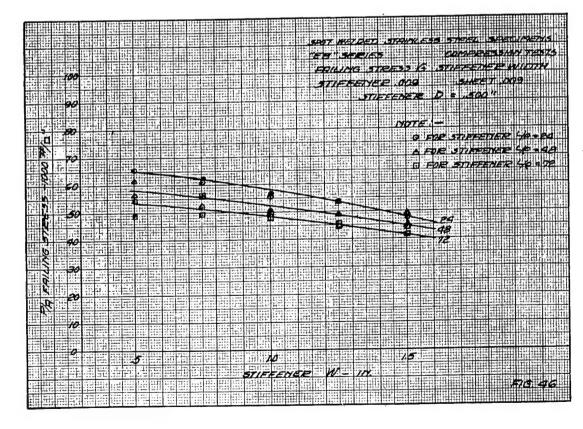


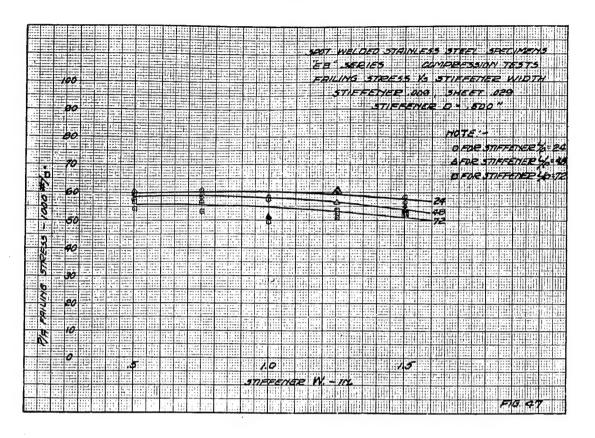


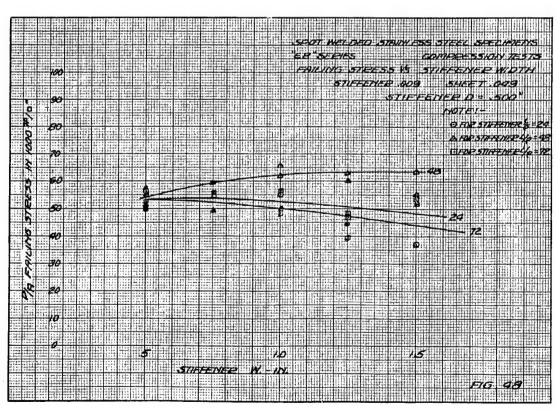


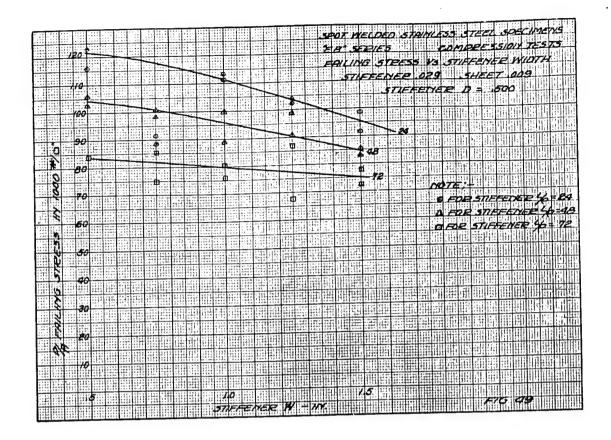


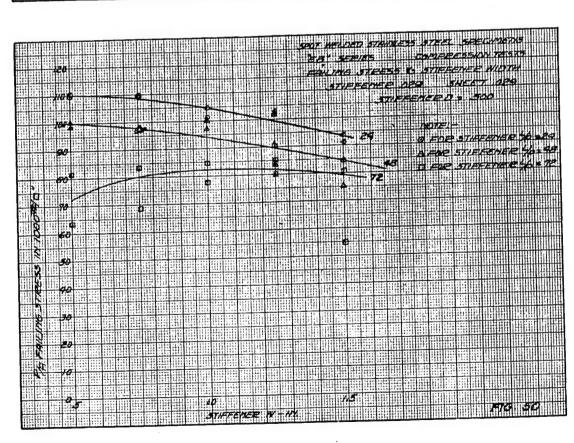


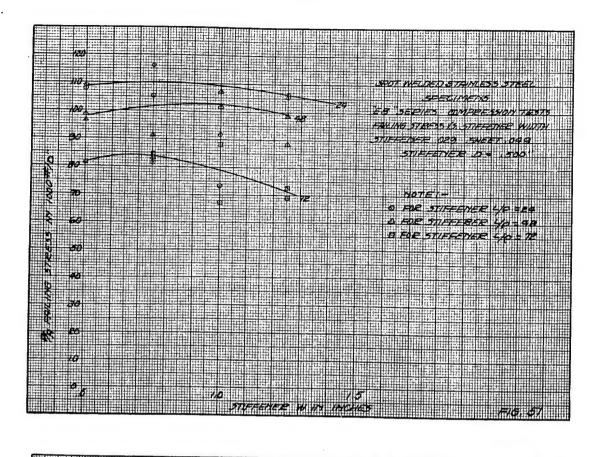


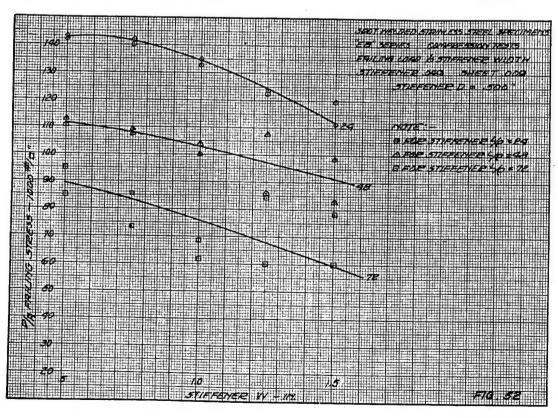


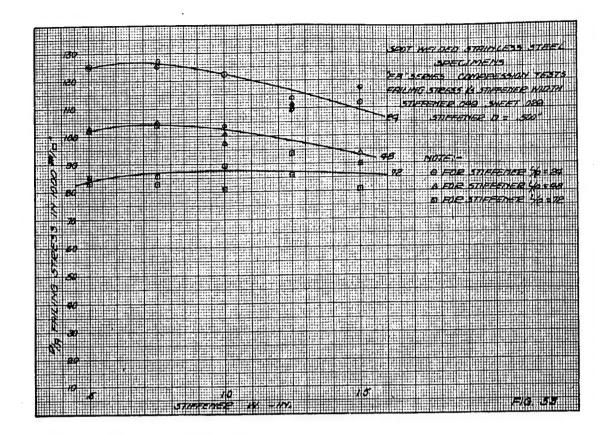


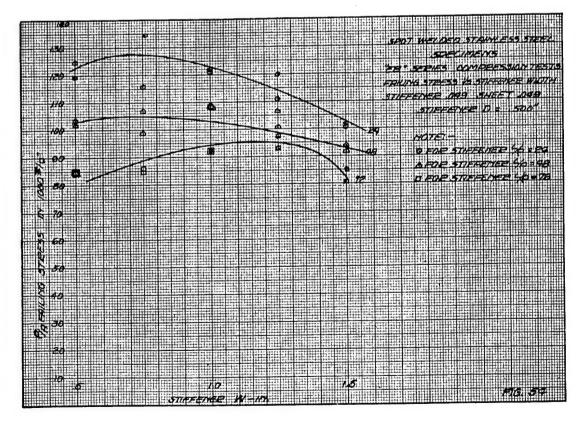


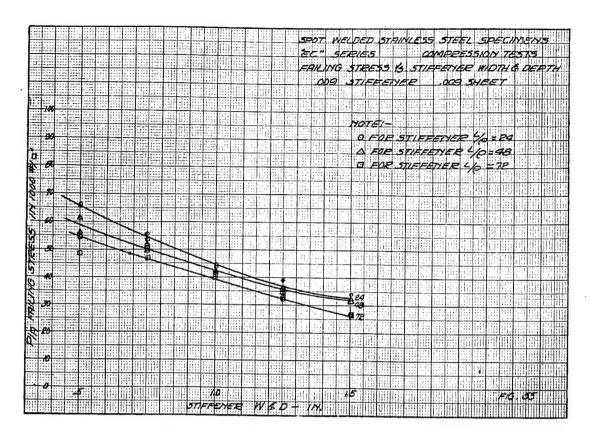


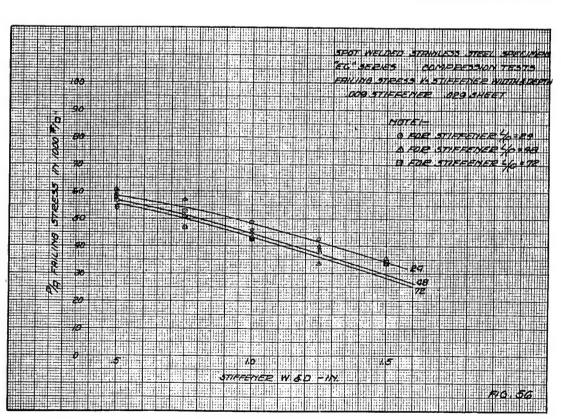


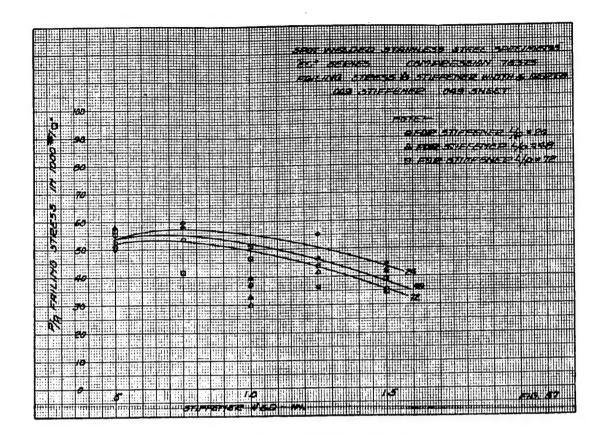


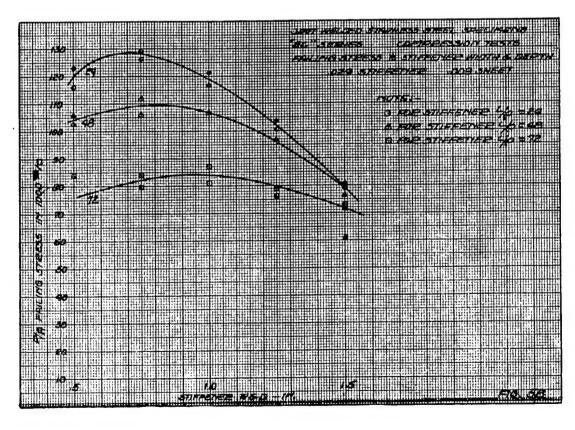


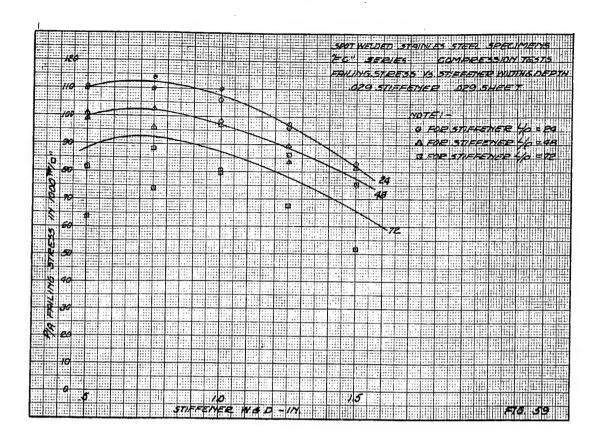


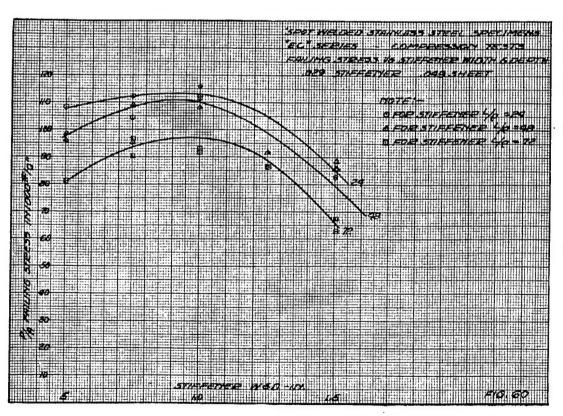


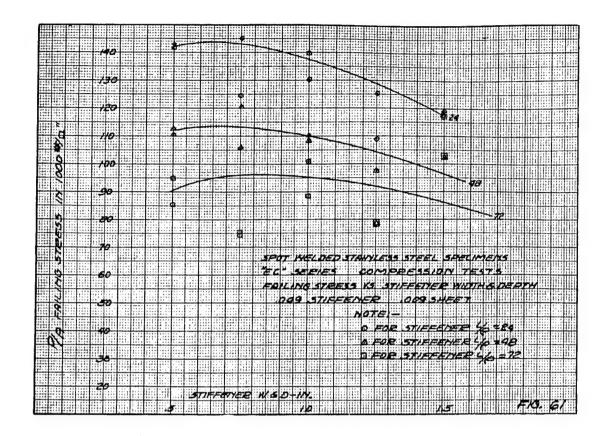


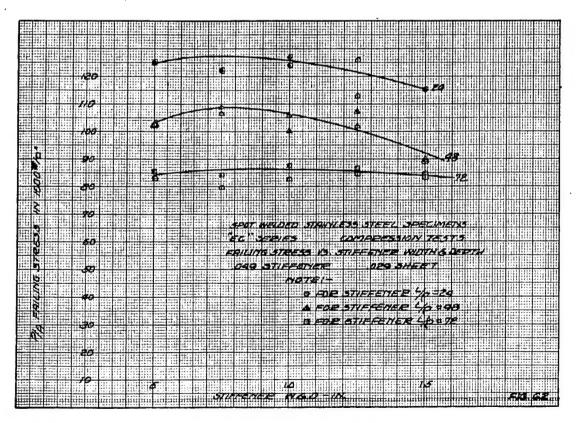


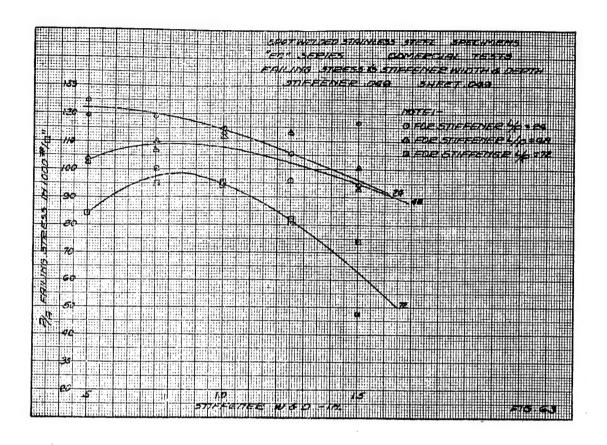


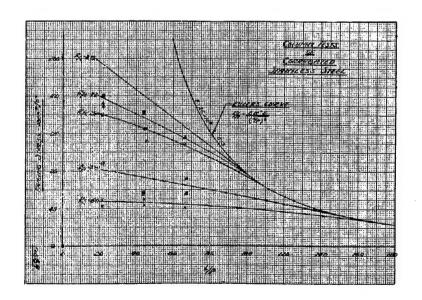


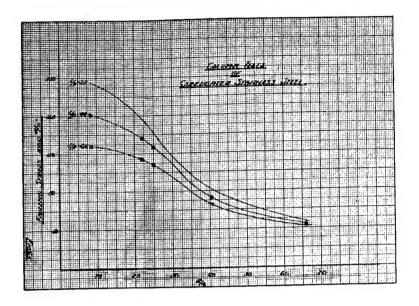


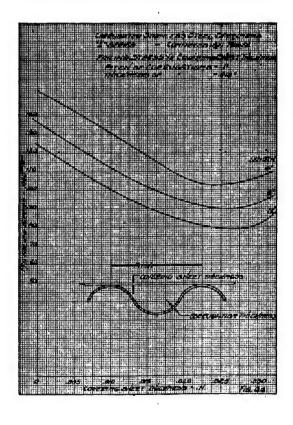


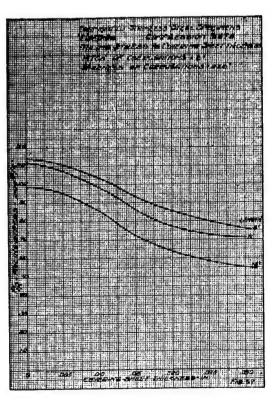


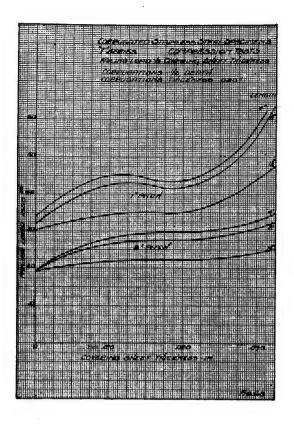


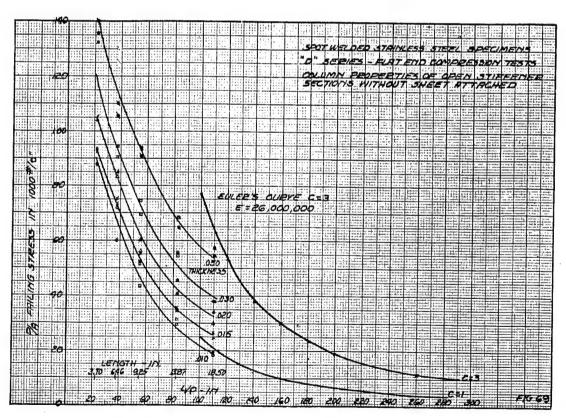


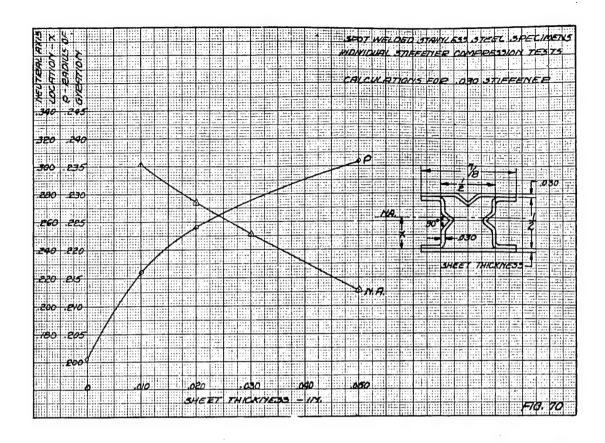


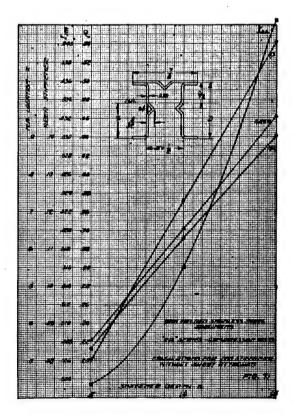


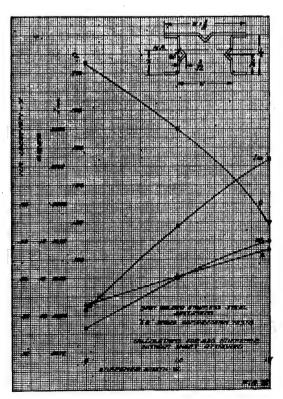


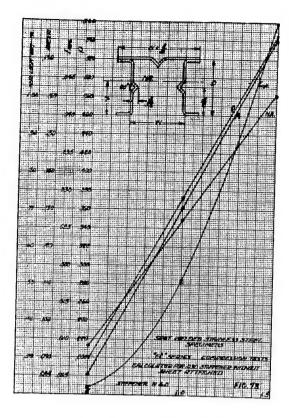












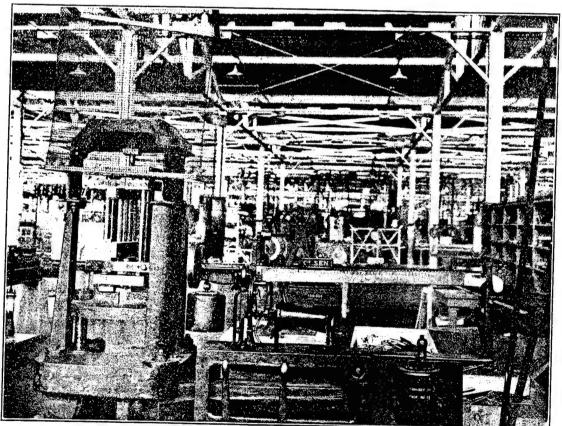


FIGURE 74.—Compression Test Set-up with Short Screws in Jig.

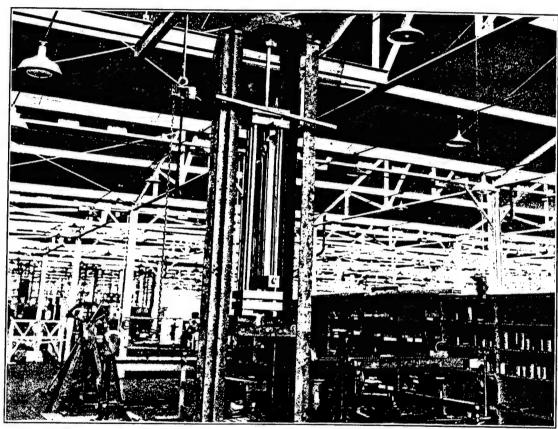


FIGURE 75.—Compression Test Set-up with Long Screws in Jig.

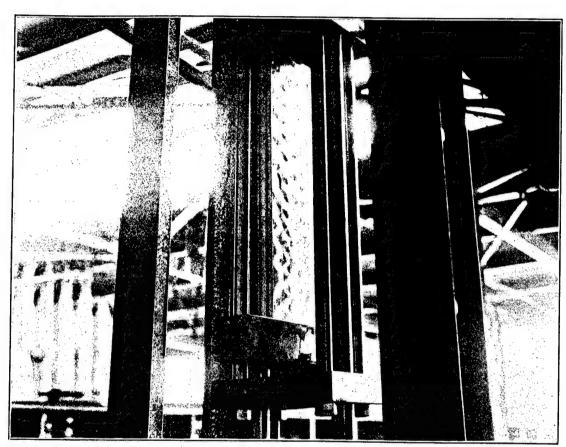
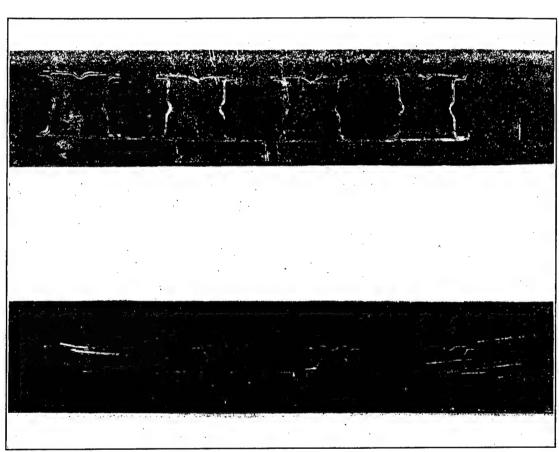


FIGURE 76.—Close-up View of Set-up of Specimen Using Long Screws in Jig.



 ${\bf Figure}\ \ {\bf 77.-Typical}\ {\bf Impression}\ {\bf of}\ {\bf Specimens}\ {\bf Left}\ {\bf on}\ {\bf Dural}\ {\bf Seating}\ {\bf Strips}.$

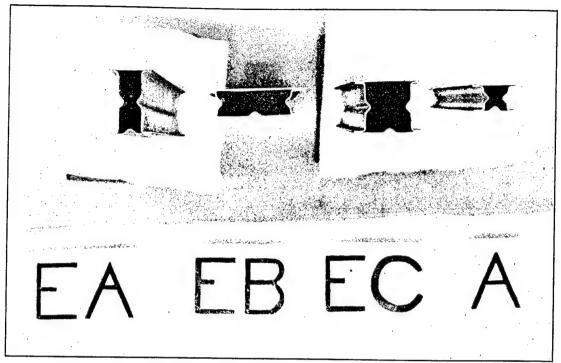


FIGURE 78.—Typical EA, EB, EC, and A Series Stiffener Sections.

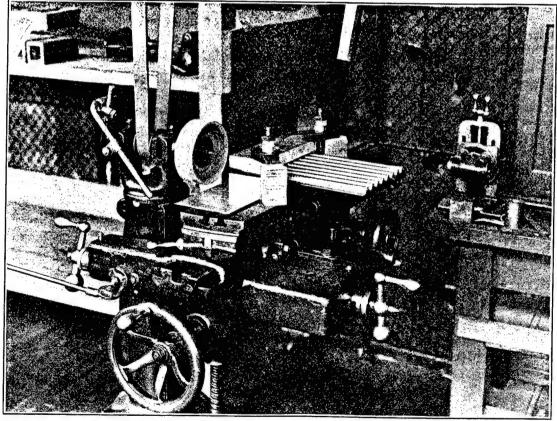


Figure 79.—Set-up Used for Regrinding the Ends on the I Series Specimens.

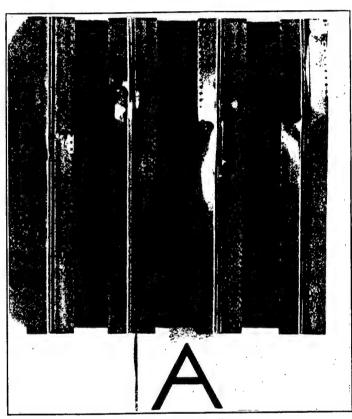


FIGURE 80.-Type A Failure.

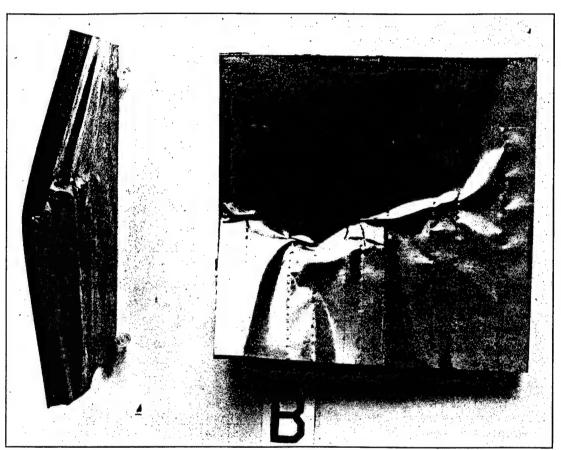


FIGURE 81.—Type B Failure.

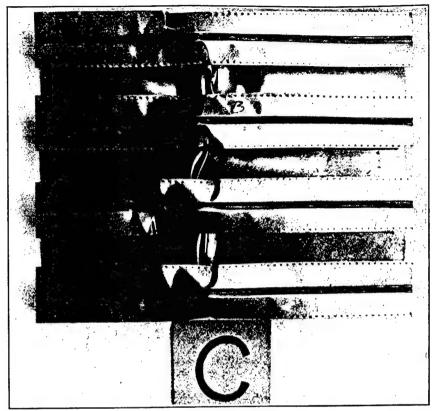


FIGURE 82.-Type C Failure.

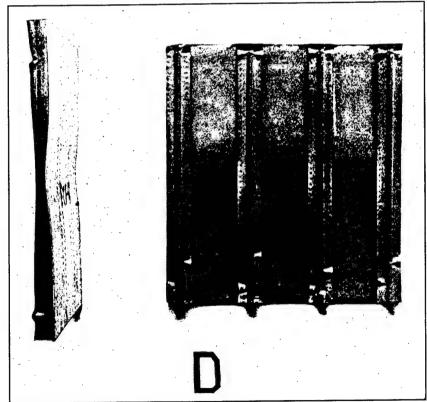


FIGURE 83.-Type D Failure.

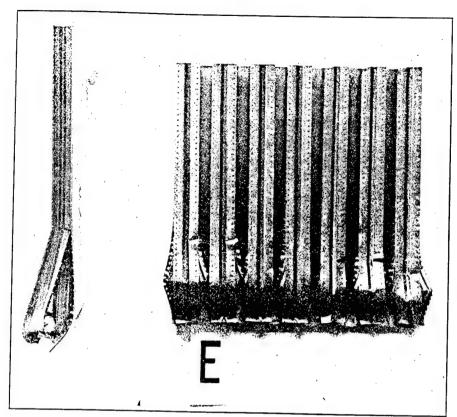


FIGURE 84. -Type E Failure.

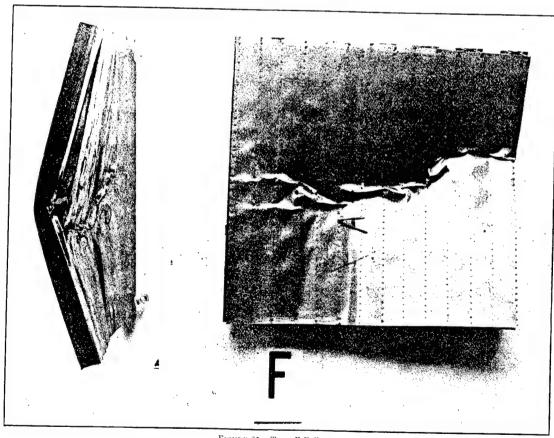


FIGURE 85. -Type F Failure.

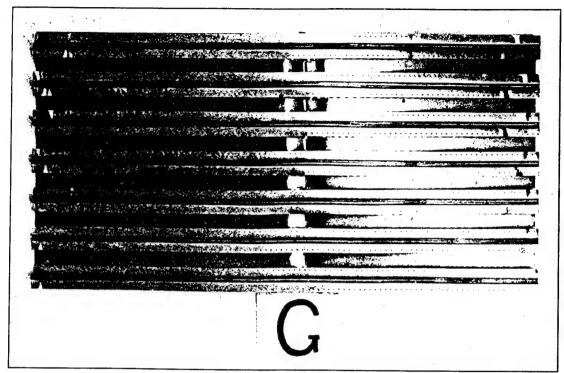


FIGURE 86, -Type G Failure.



FIGURE 87.-Type H Failure.

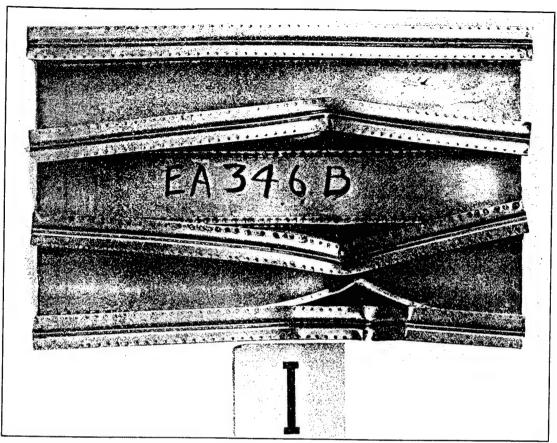


FIGURE 88.-Type I Failure.

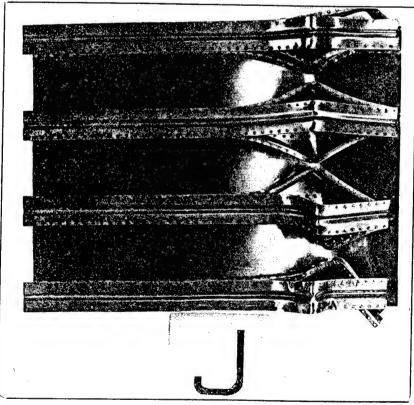


FIGURE 89.—Type J Failure.

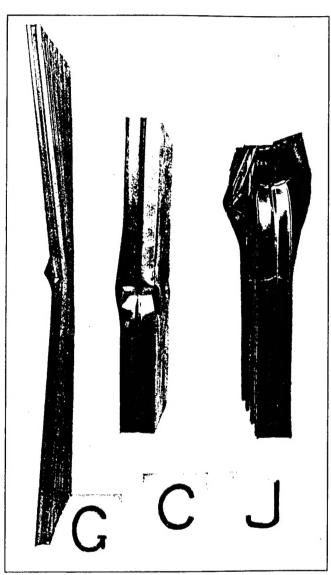


FIGURE 90.—Side View of Type C, G, and J Failures.

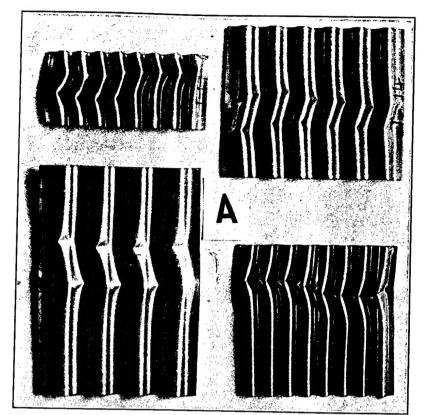


FIGURE 91.—Buckling Failure of Corrugated Sheet.

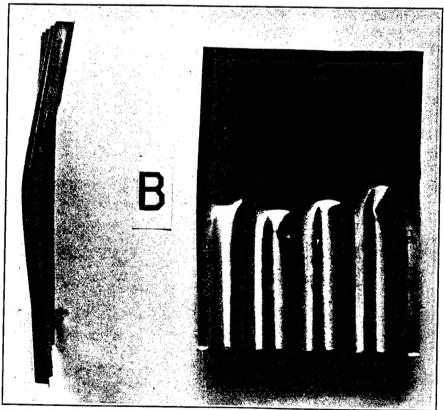


FIGURE 92.—Column Failure of Corrugated Sheet.

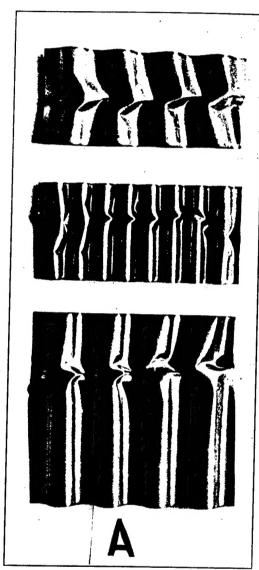


FIGURE 93.—Buckling of Corrugated Sheet with Flat Sheet Attached.

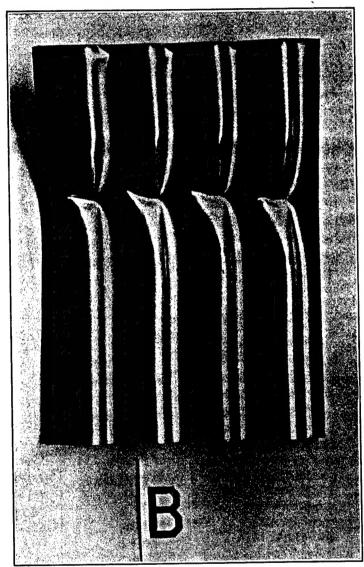


FIGURE 94.—Column Failure of Corrugated Sheet with Flat Sheet Attached.